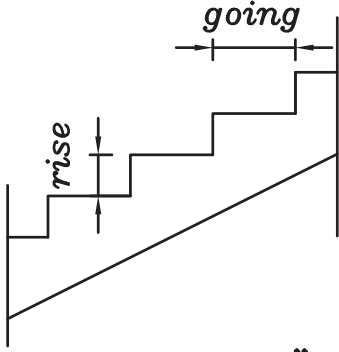


STAIRS

-هى بلاطات مائلة للتوصيل من منسوب الى منسوب اخر وتكون مزودة بدرج
لامكانية الصعود عليها وعادة تكون بلاطة السلم اما

(one way or cantilever solid slab)



-وتتراوح ابعاد (rise) من (13-16 cm) وعادة تؤخذ

(15cm) وتتراوح ابعاد (going) من (26-30 cm)

وعادة تؤخذ (30cm) .

- أكبر عدد من الدرجات فاللحة الواحدة يساوى (١٤) درجة

$$\text{No. of steps} = \frac{\text{difference between levels}}{\text{Rise height (0.15)}}$$

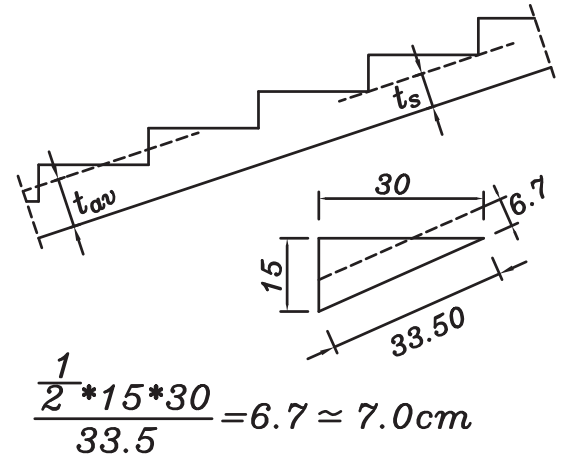
$$\text{Length of stair} = \text{No. of steps} * 0.30$$

Concrete dimensions:

$$t_s = \frac{L_s}{25, 30, 36} \quad (\text{One way slab})$$

$$t_s = \frac{L_c}{10} \quad (\text{cantilever slab})$$

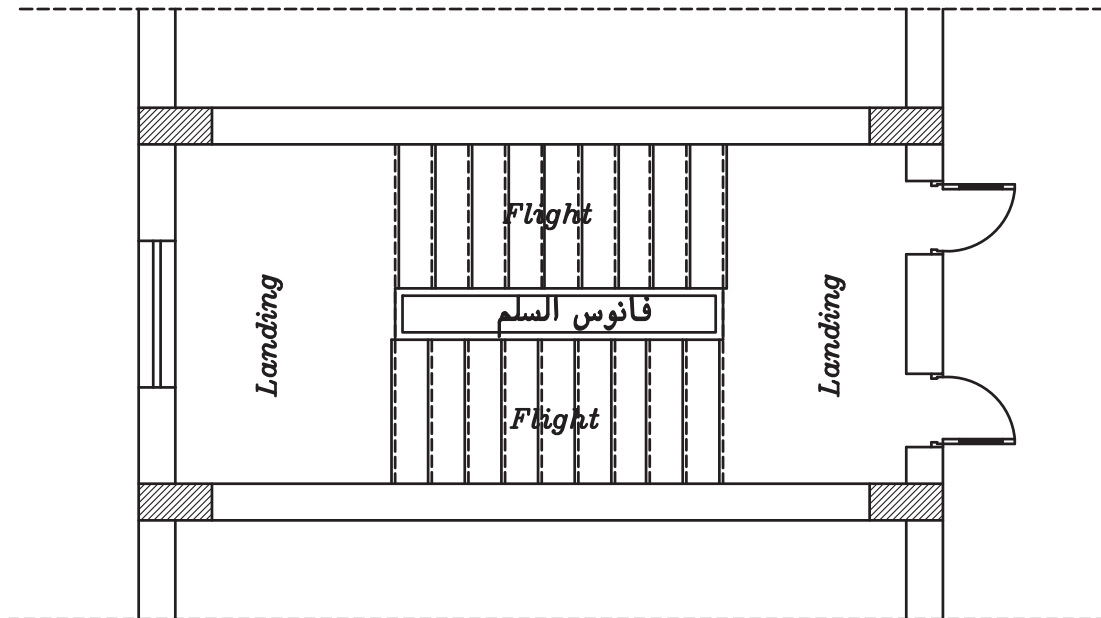
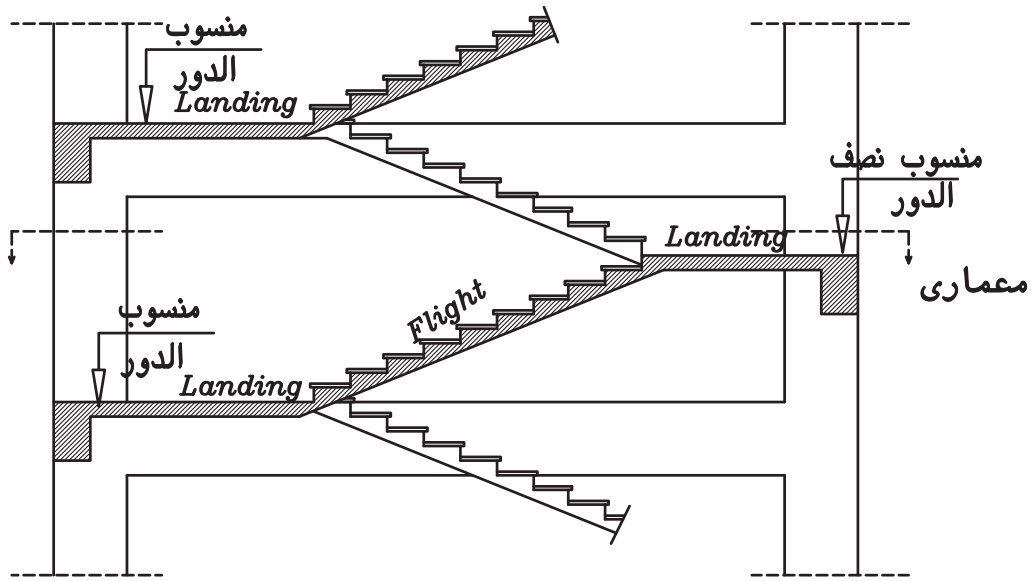
$$t_{av} = t_s + 7 \text{ cm}$$



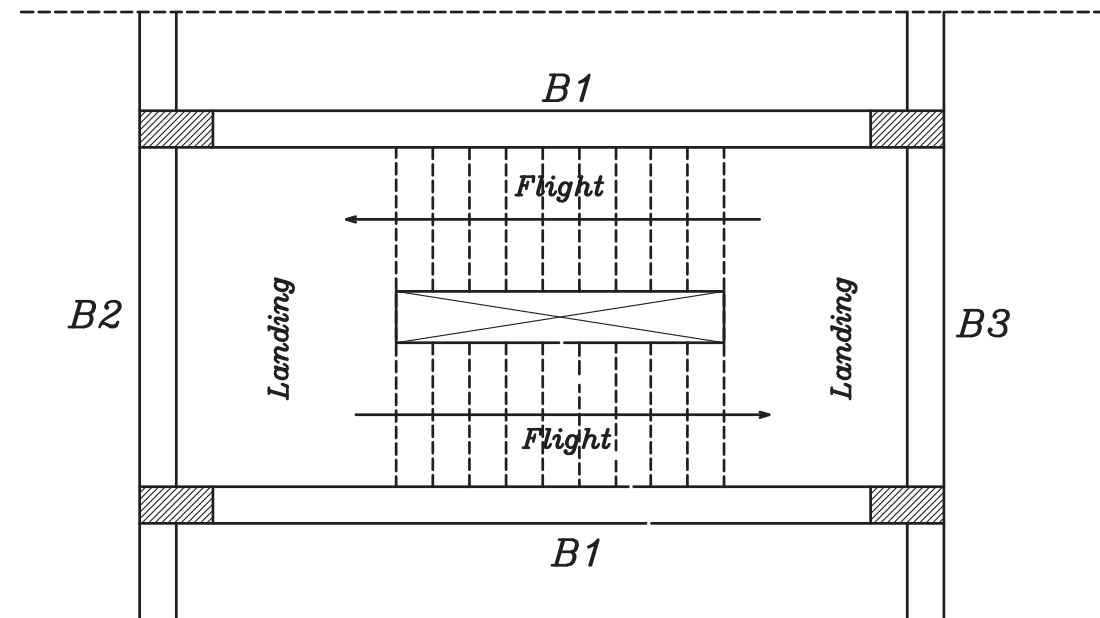
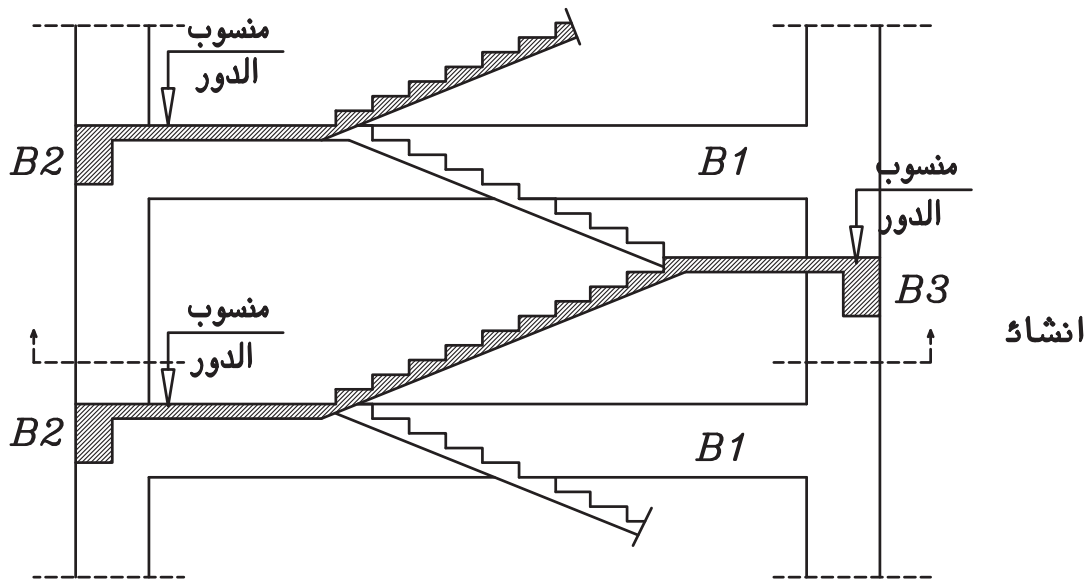
$$w_{su} = 1.4(t_{av} \gamma_c + F.C.) + 1.6L.L. \cos \theta \quad \text{kN/m}^2$$

- ملحوظة

لاحظ ان (t_s) هى التى تستخدم فى التصميم بينما (t_{av}) تستخدم فى حساب
الاحمال فقط .



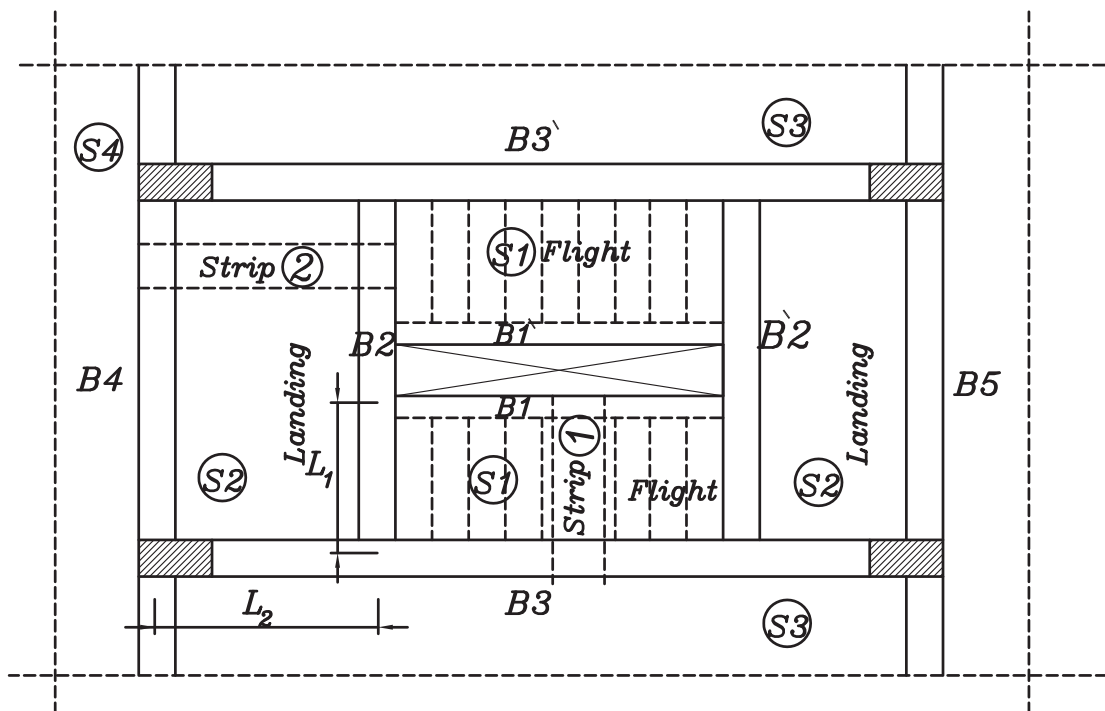
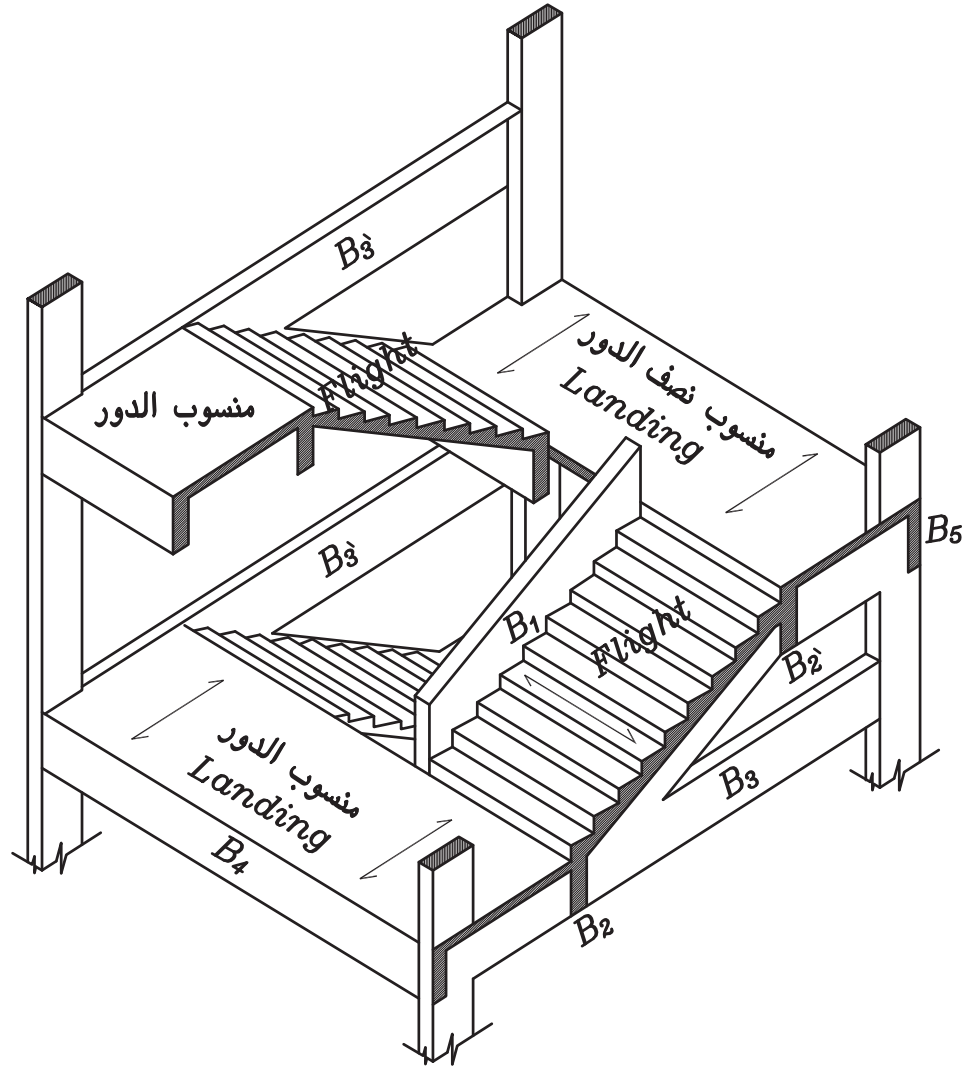
Arc. Plan



Structural Plan

Systems of Stairs used in ordinary buildings

Type(1)



1-Slabs

Calculate t_s

$$t_{av} = t_s + 7\text{cm}$$

$$w_{su} = 1.4[t_s \gamma_c + F.C.] + 1.6L.L.$$

$$w_{su} = 1.4[t_{av} \gamma_c + F.C.] + 1.6L.L. \cos \theta$$

Strip (1)

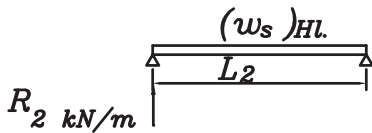


شريحة افقية فى بلاطة مائلة

$$M_{des.} = M \cos \theta$$

$$d = t_s - 20\text{mm}$$

Strip (2)

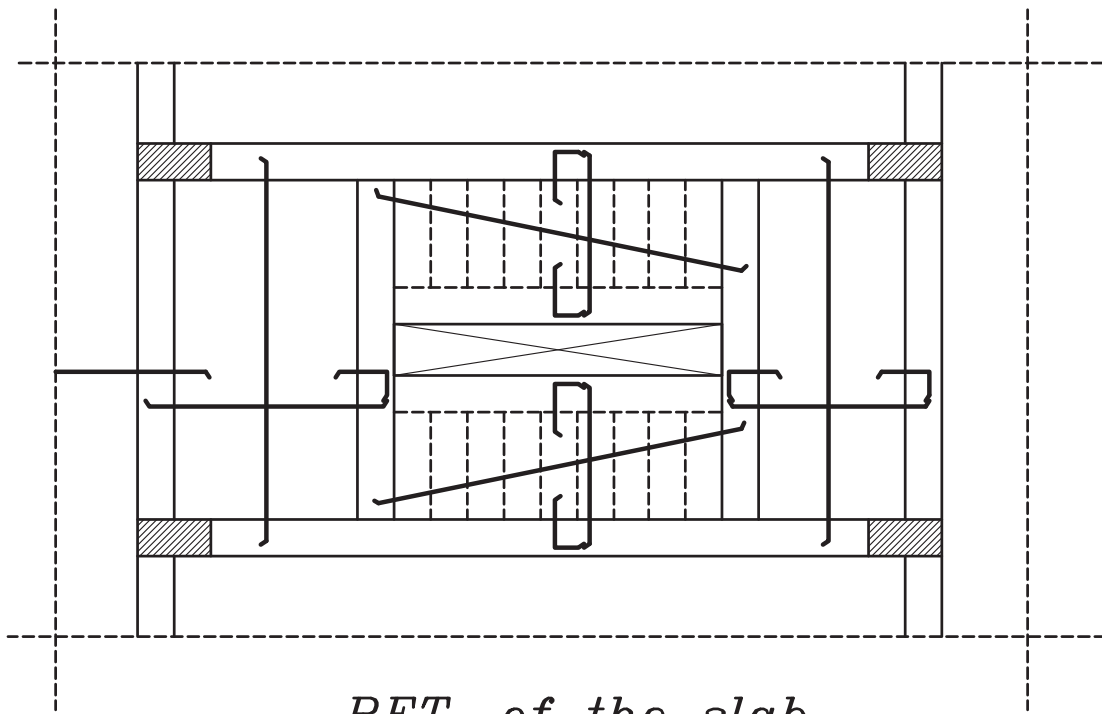


$$M_{des.} = M$$

$$d = t_s - 20\text{mm}$$

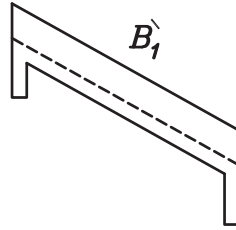
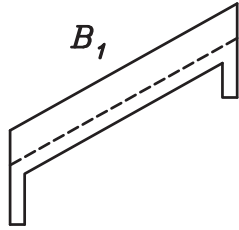
ملحوظة

نصمه شريحة السلم على انها *simple span* وذلك لسهولة الحل والتنفيذ.



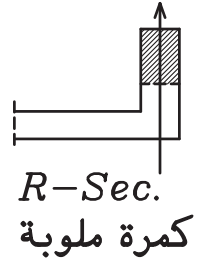
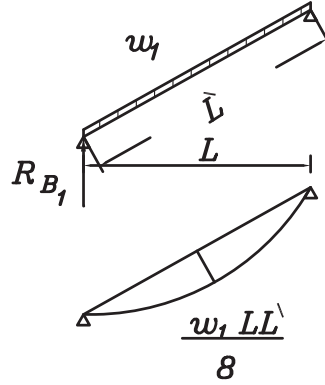
2-Beams

B_1, B_1'



$$w_1 = 0.w.+ R_1$$

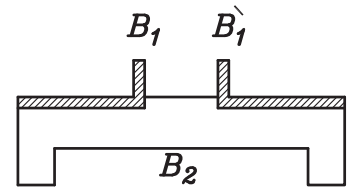
$$w_1 = \gamma_c b(t-t_s) * 1.40 + R_1$$



B_2, B_2'

$$w_2 = 0.w.+ R_2$$

$$w_2 = \gamma_c b(t-t_s) * 1.40 + R_2$$



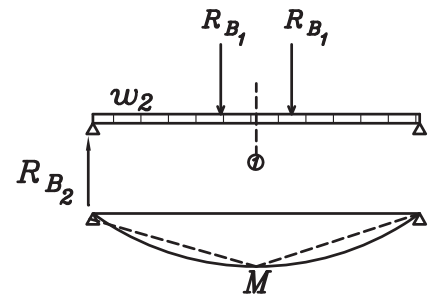
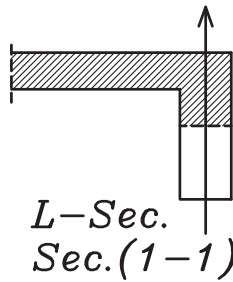
L-Sec.

C.L. - C.L.

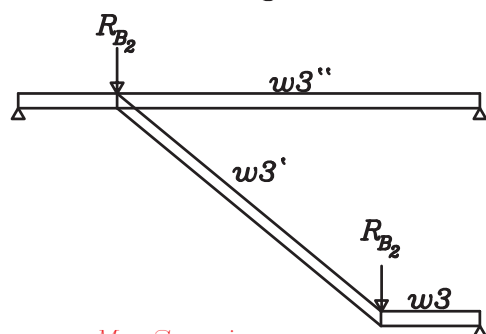
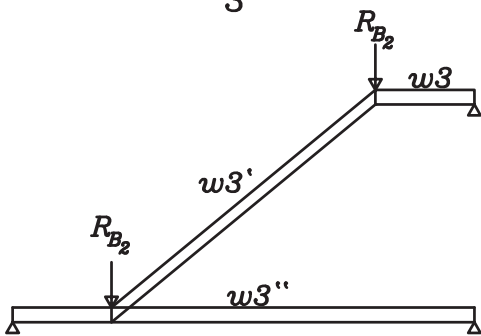
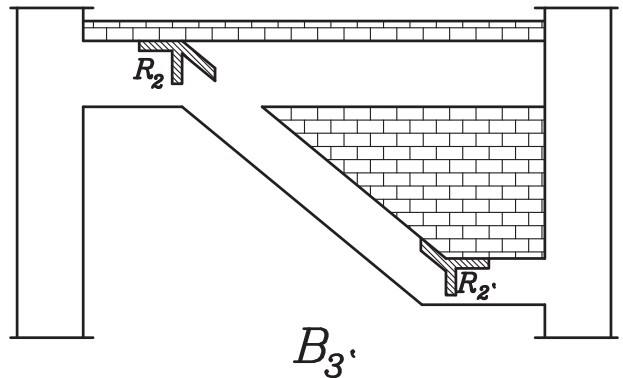
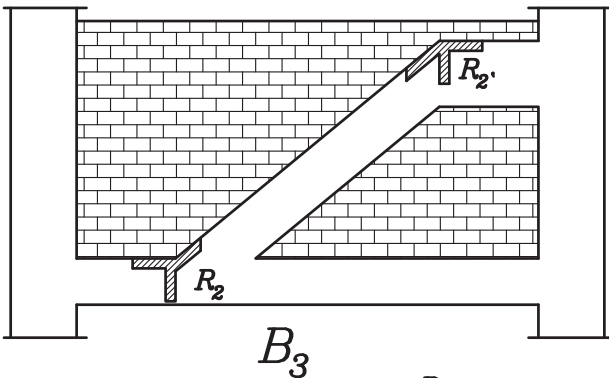
$$B = 6t_s + b$$

$$K \frac{L}{10} + b$$

الاقل



B_3, B_3'



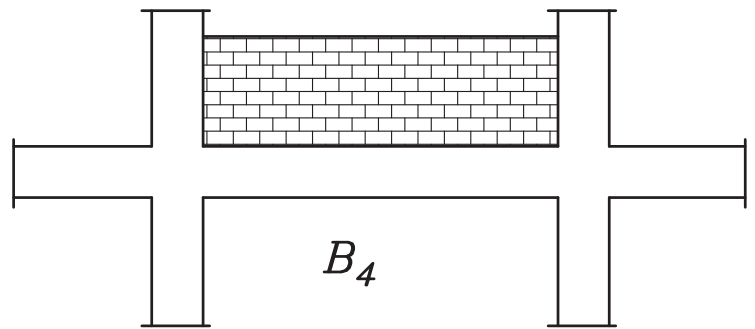
$$w_3 = o.w. + walls$$

$$w_3' = o.w. + walls + R_1$$

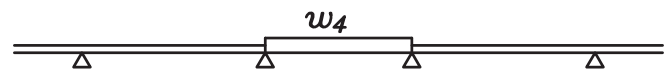
$$w_3'' = o.w. + walls + S_3$$

$$\underline{\underline{B_4}}$$

$$w_4 = o.w. + walls + R_2 + S_4$$

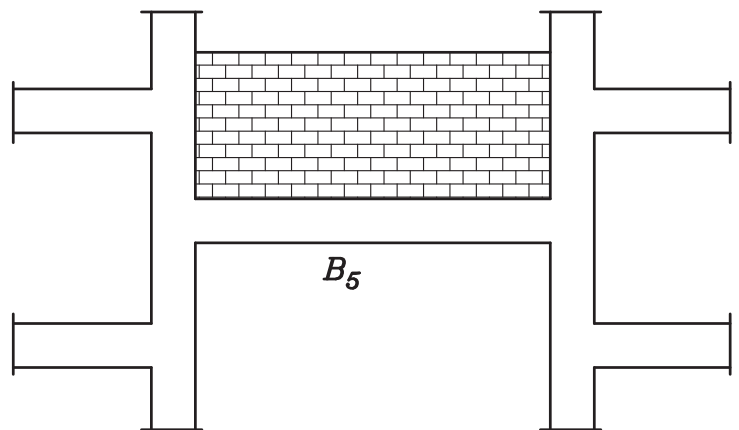


Continuous Beam.

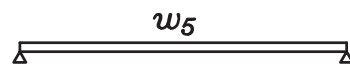


$$\underline{\underline{B_5}}$$

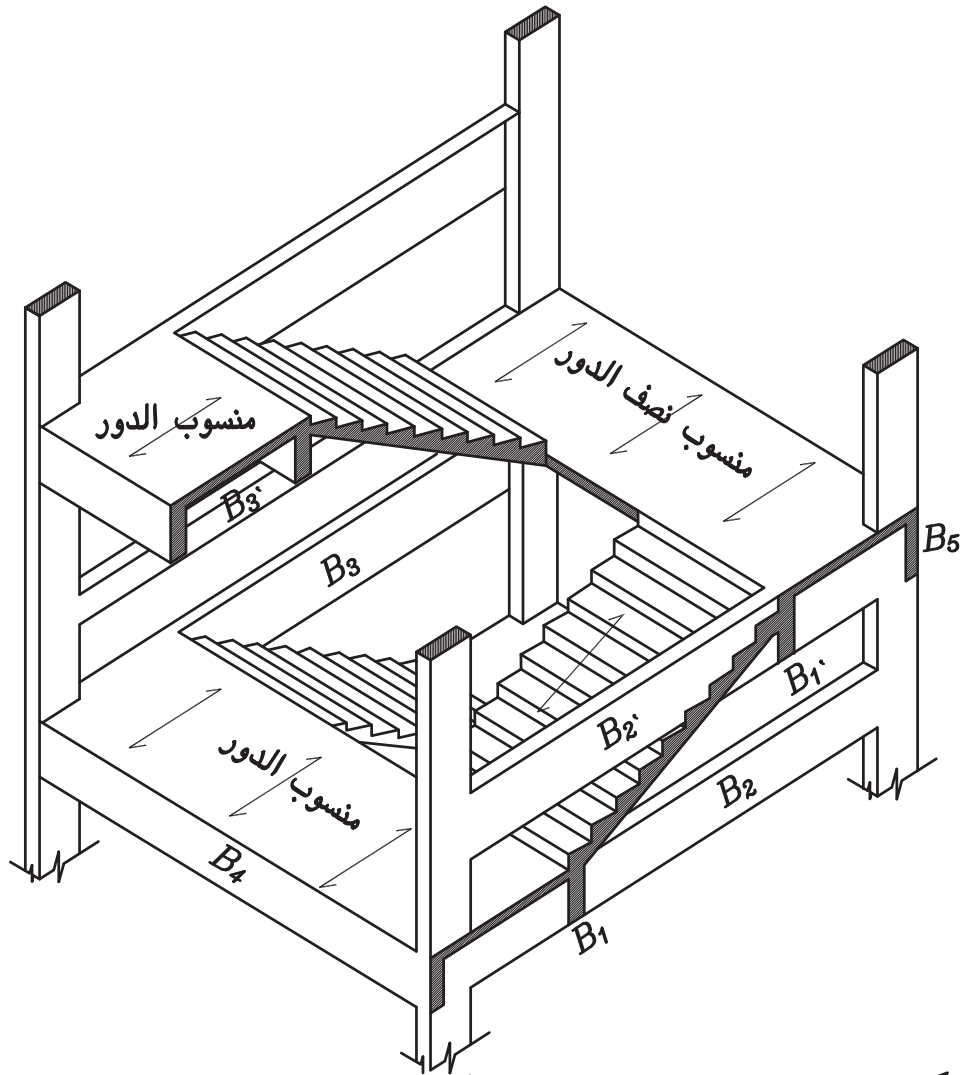
$$w_5 = o.w. + walls + R_2$$



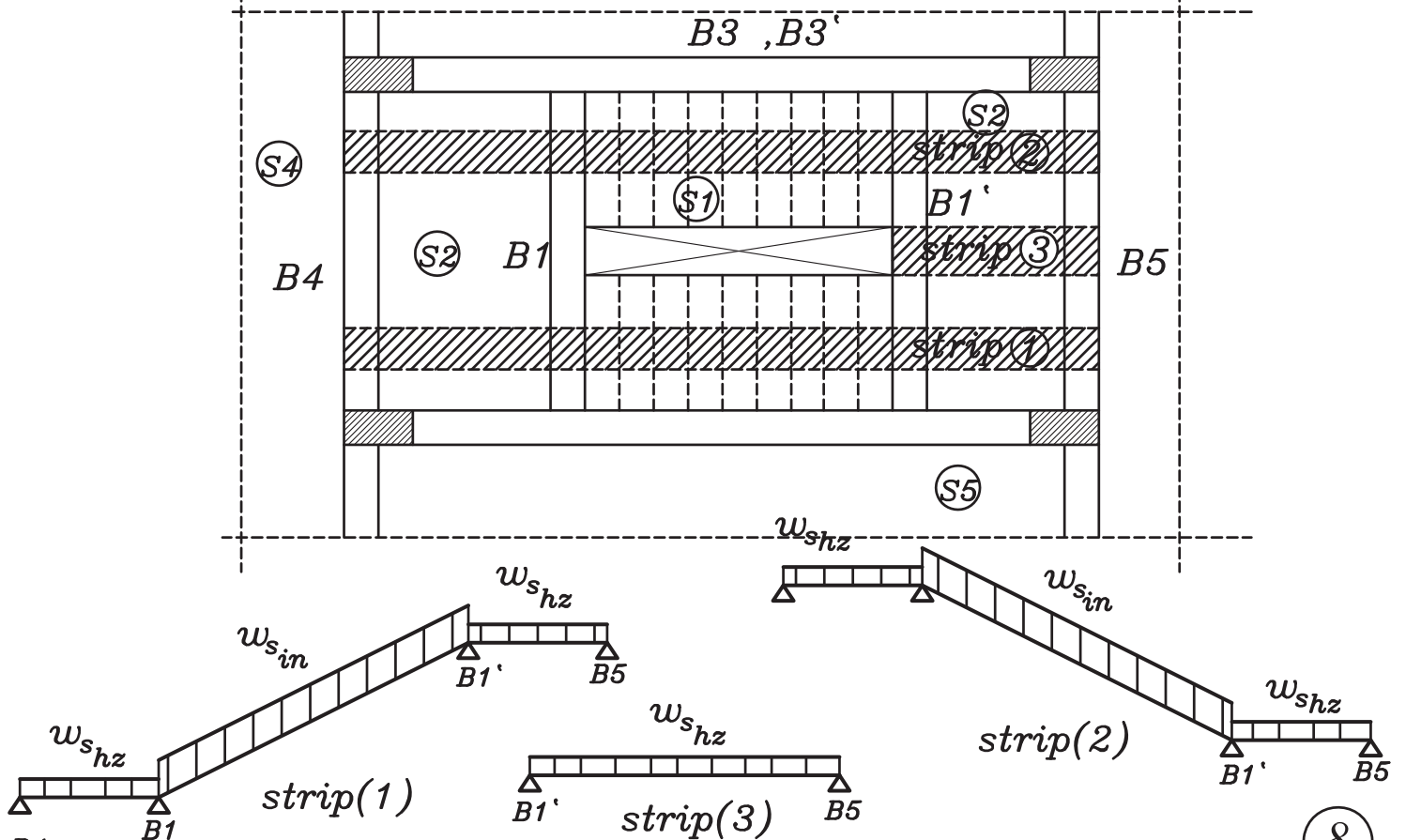
B₅ Simple Beam.



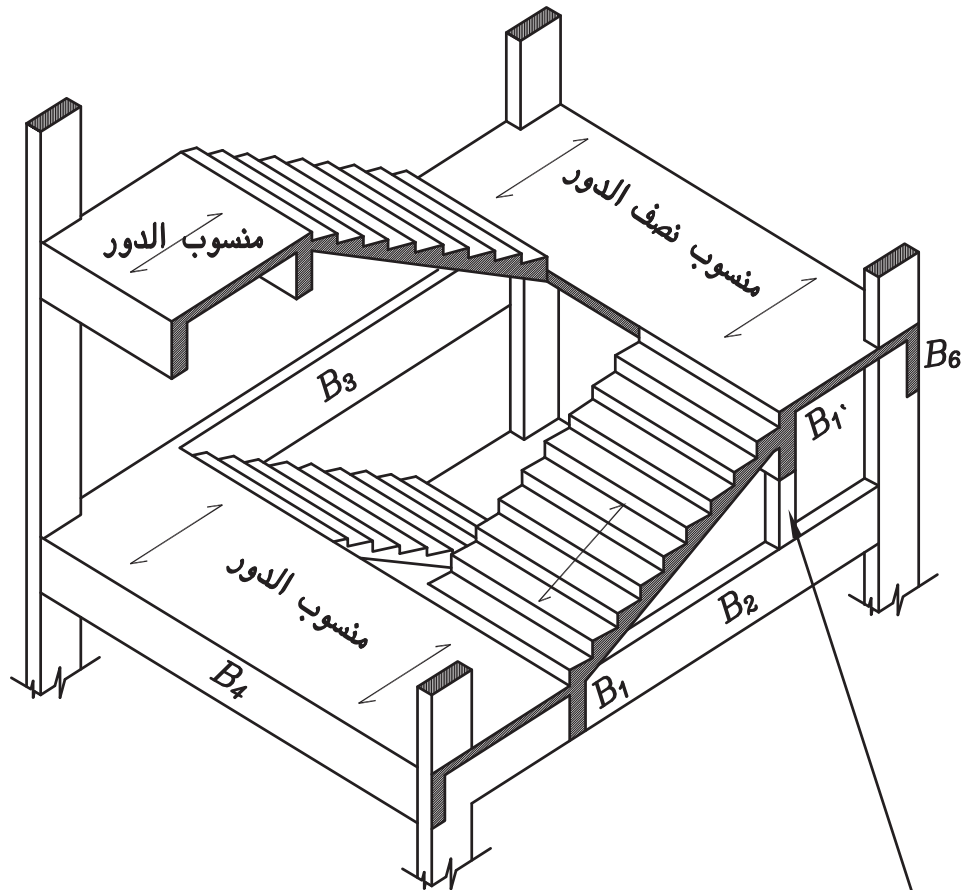
Type(2)



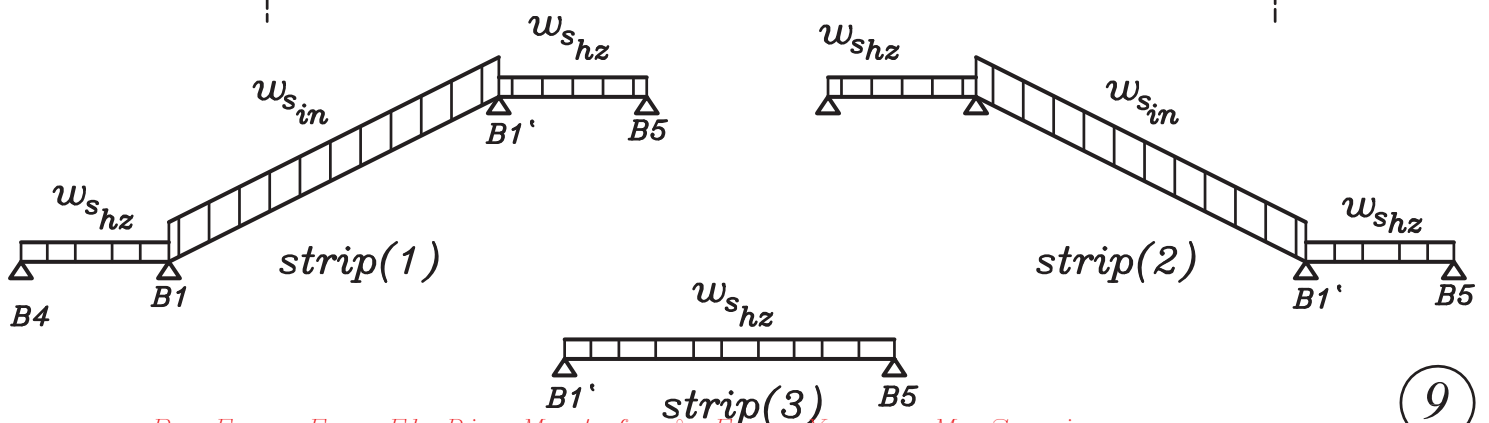
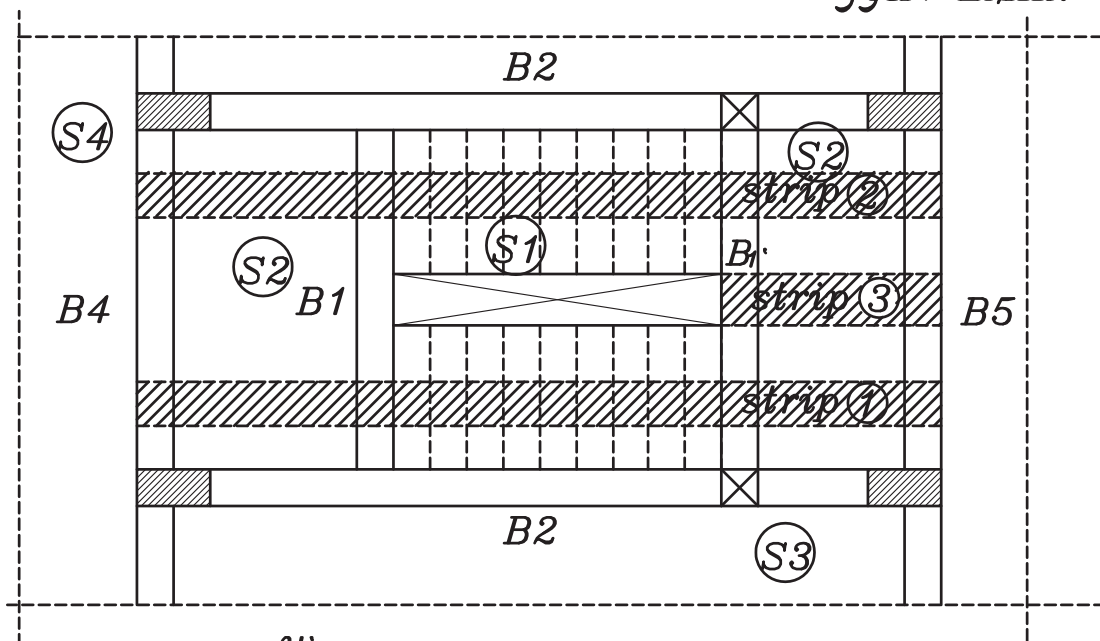
تم وضع الكمرات $B2'$ & $B3'$ لحمل الكمرة $B1'$ الواقعة في منتصف الدور



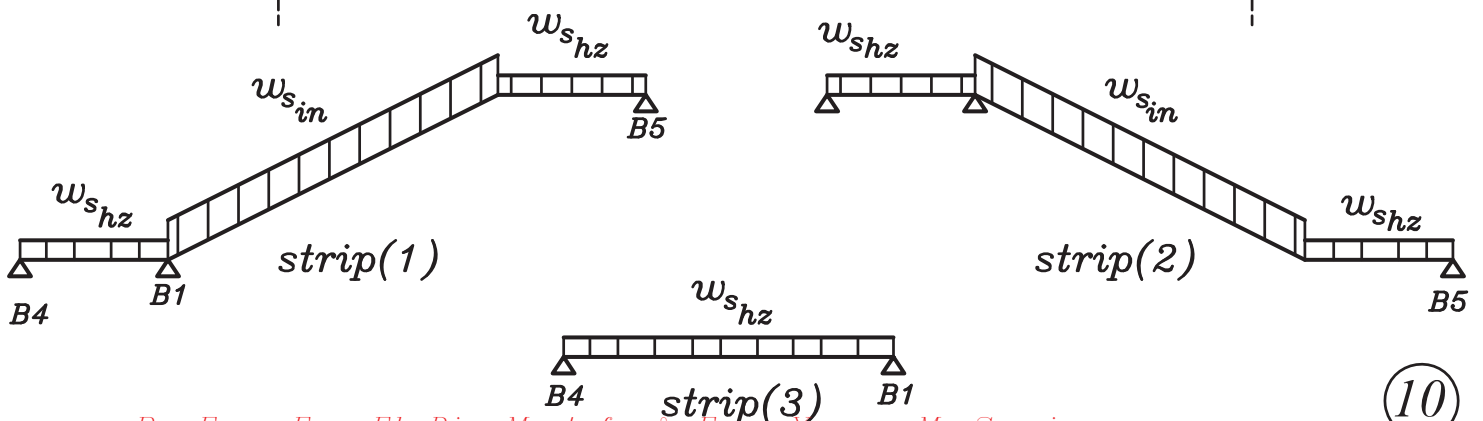
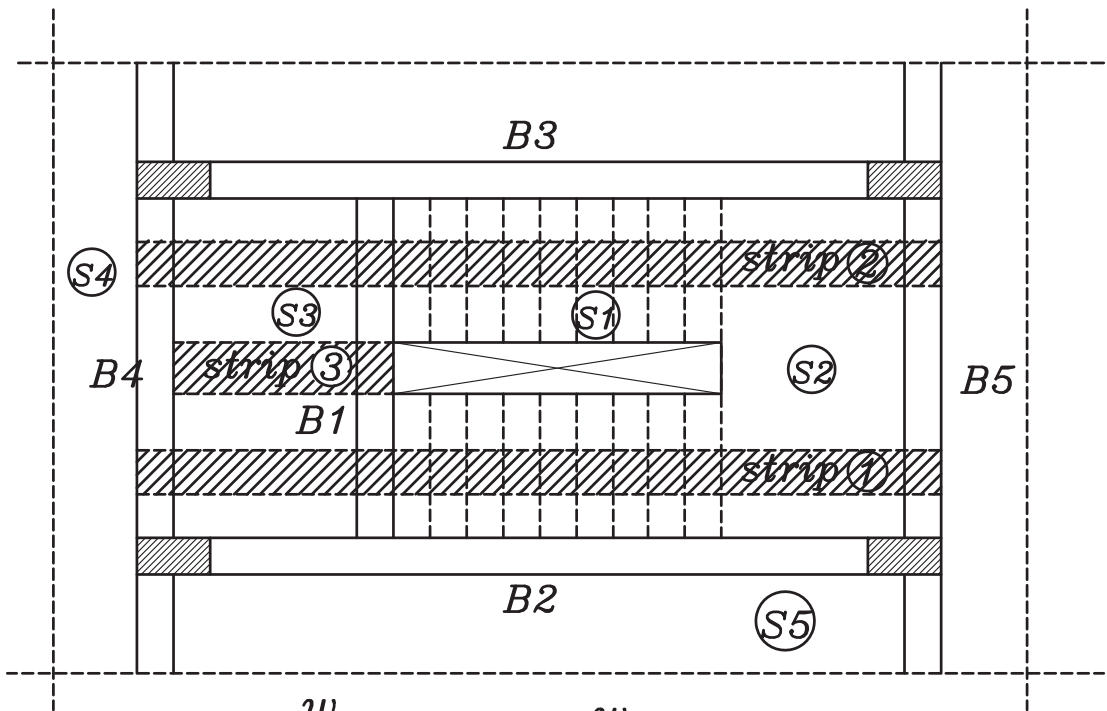
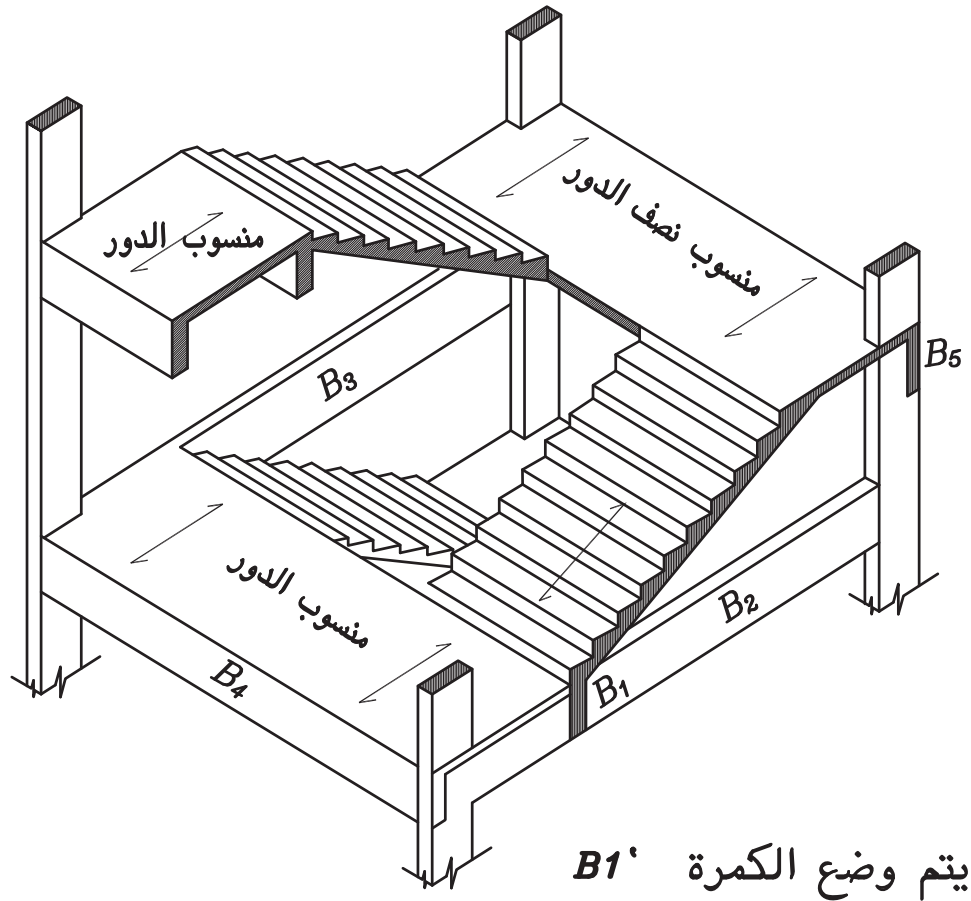
Type(3)



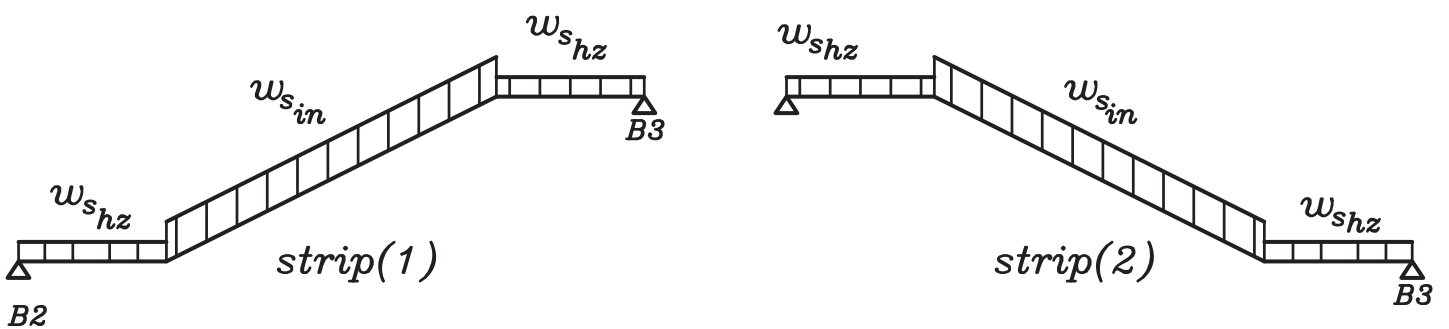
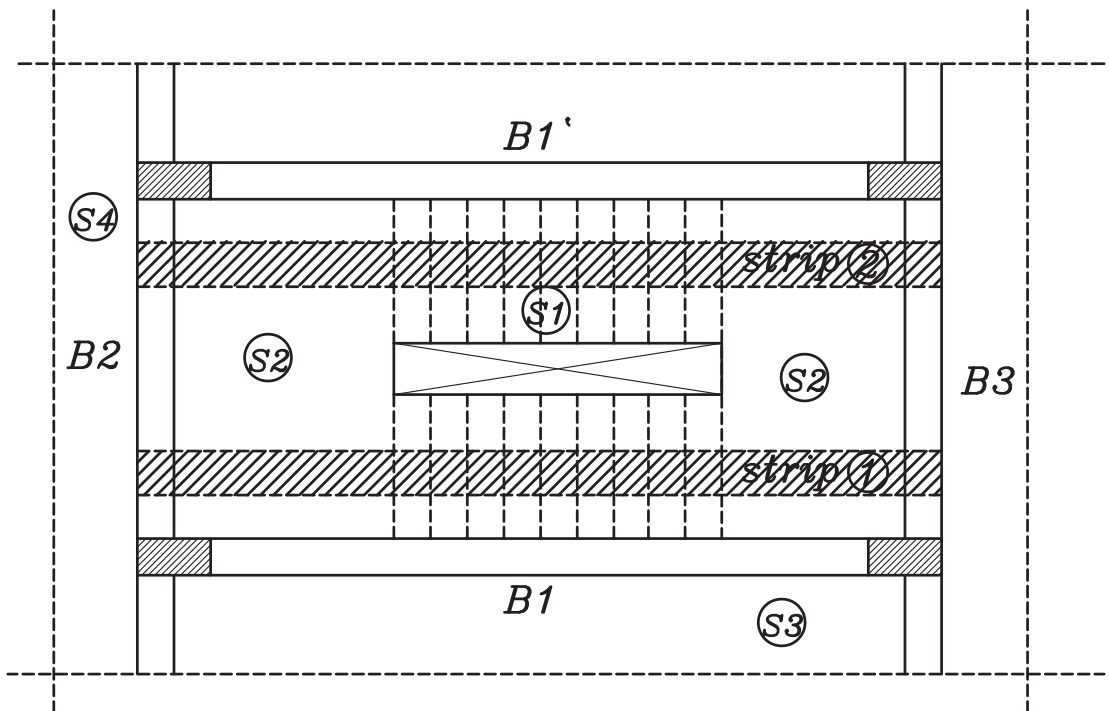
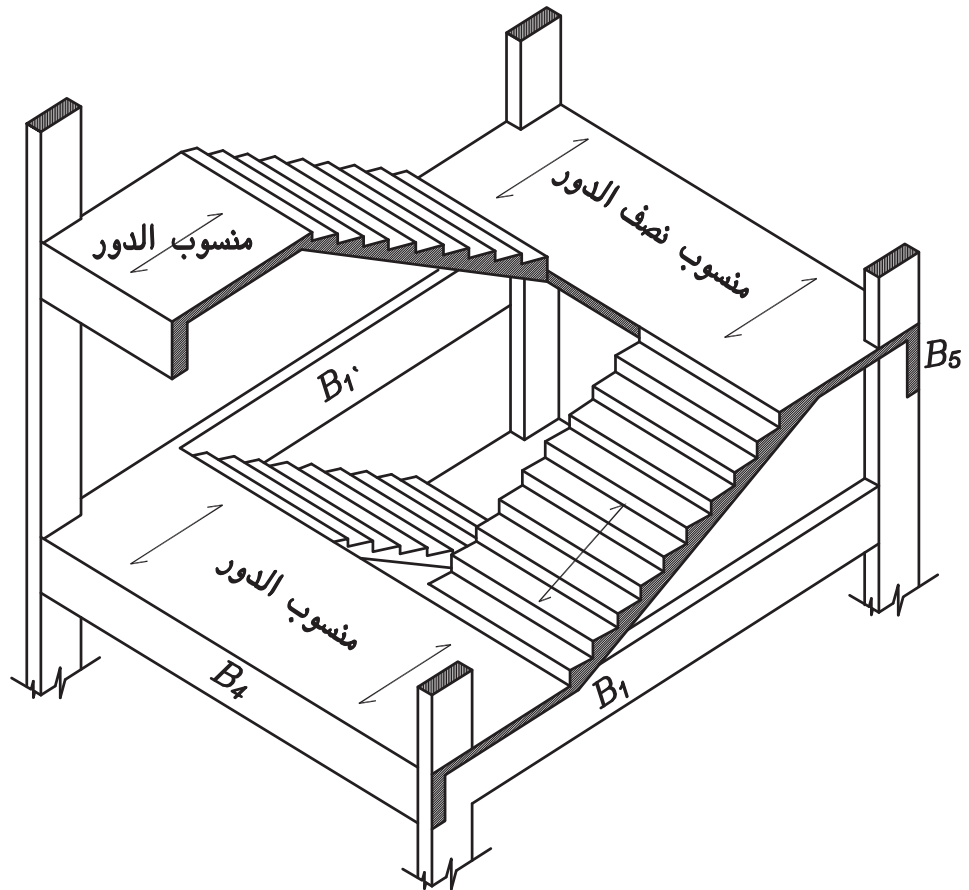
تم وضع *Post* بدلا من الكمرات $B2'$ & $B3'$ وذلك لحمل الكمرة $B1'$ الواقعة في منسوب منتصف الدور



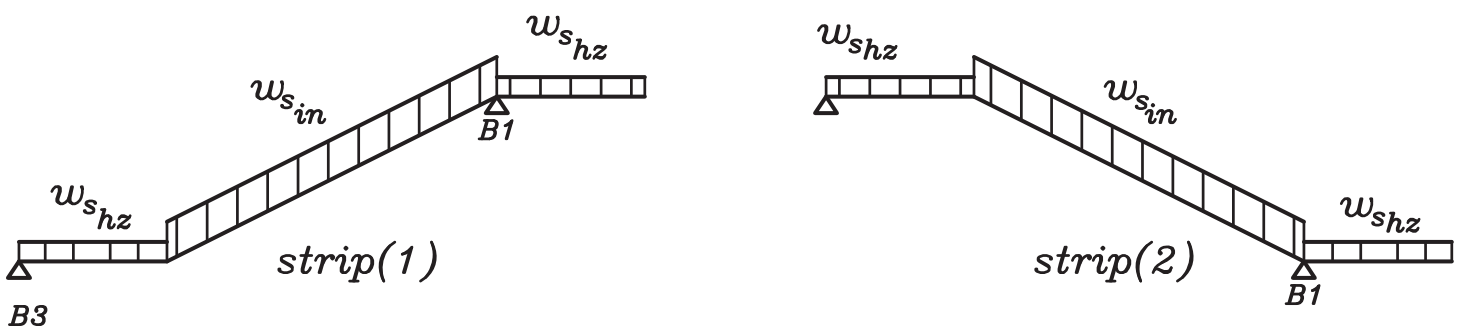
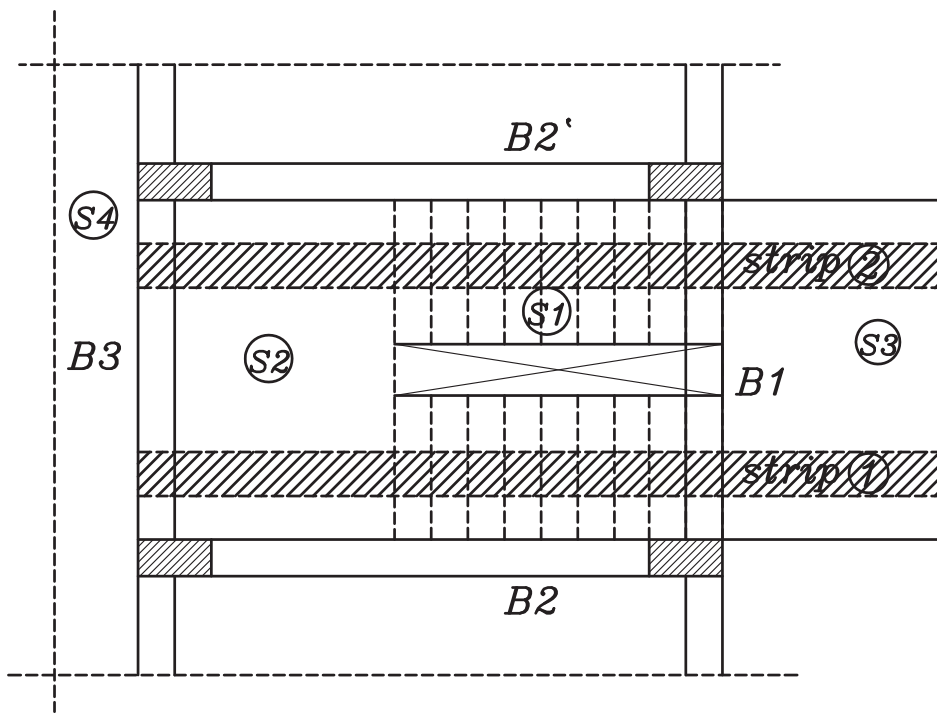
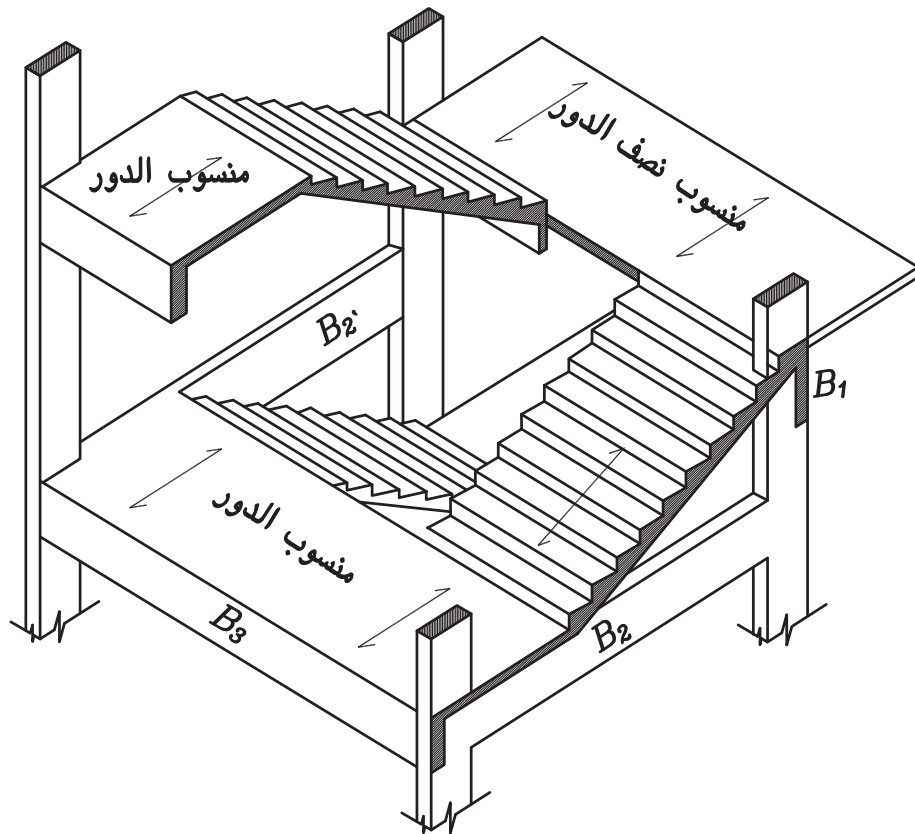
Type(4)



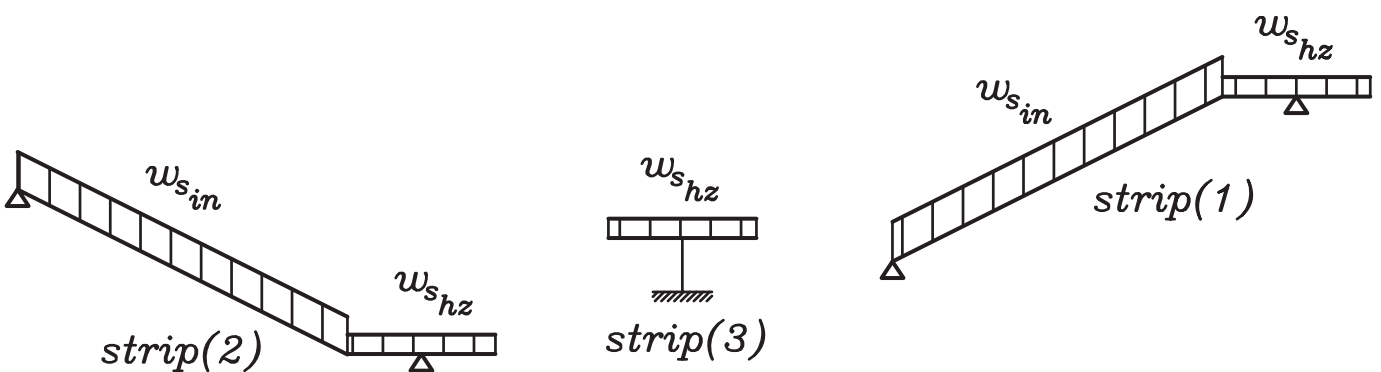
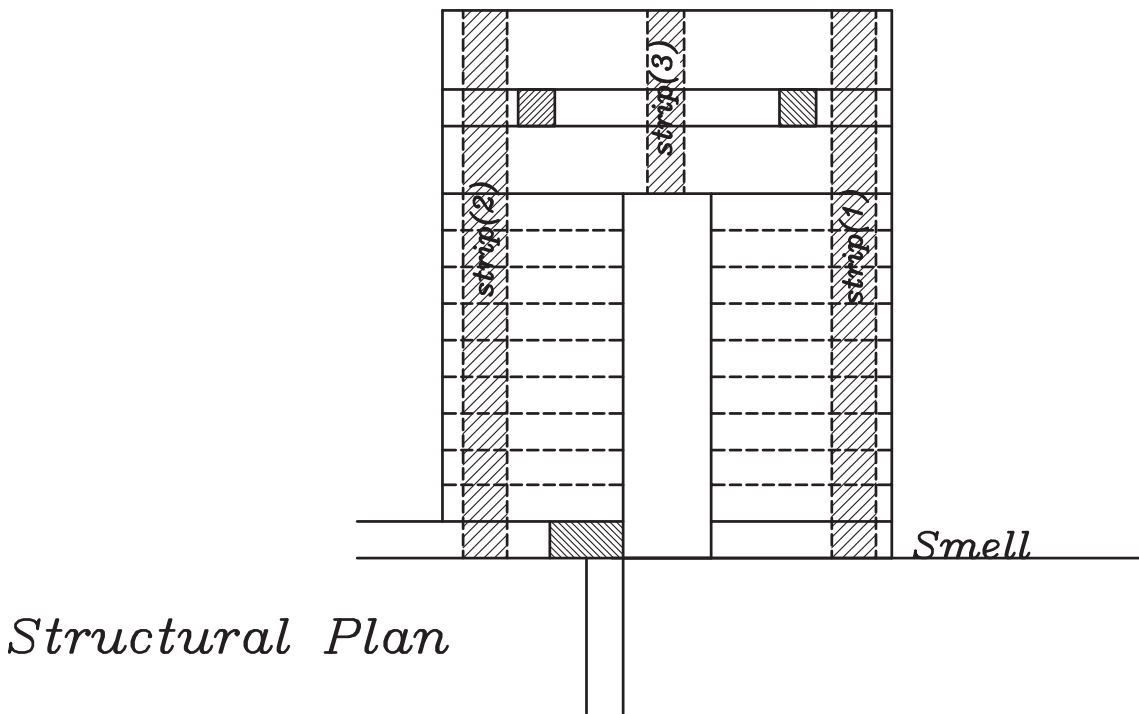
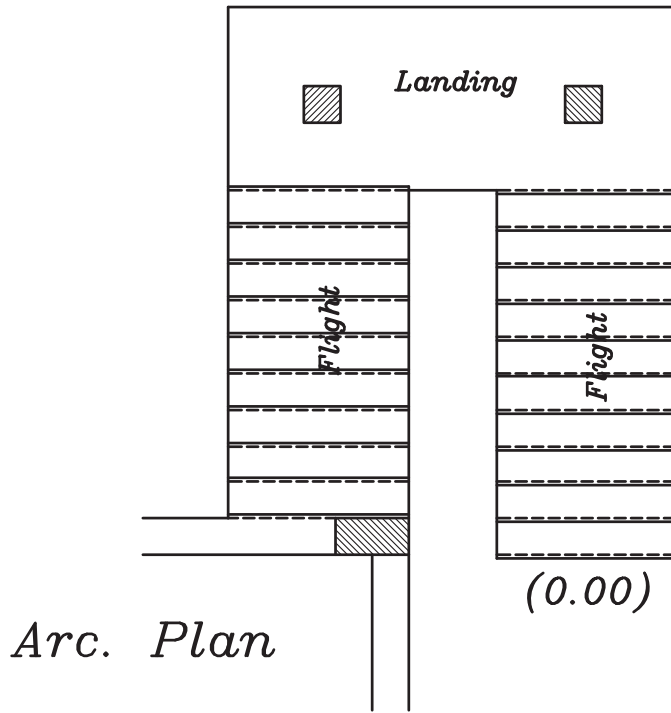
Type(5)



Type(6)

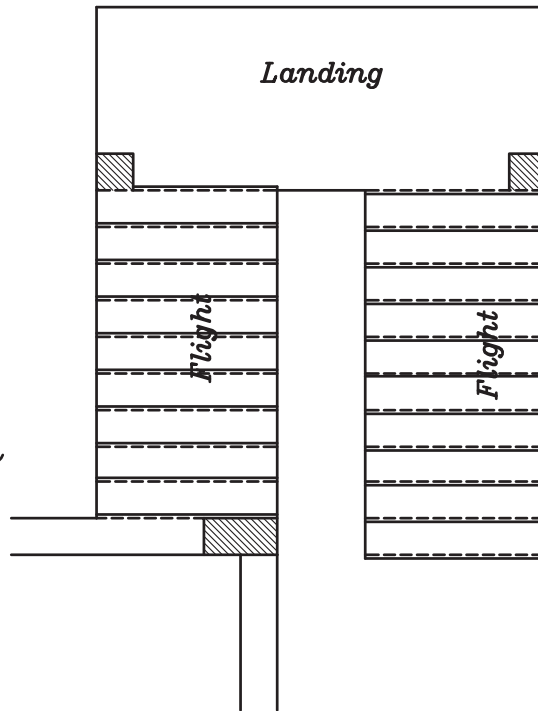


Example 1:- Show How to solve this stair

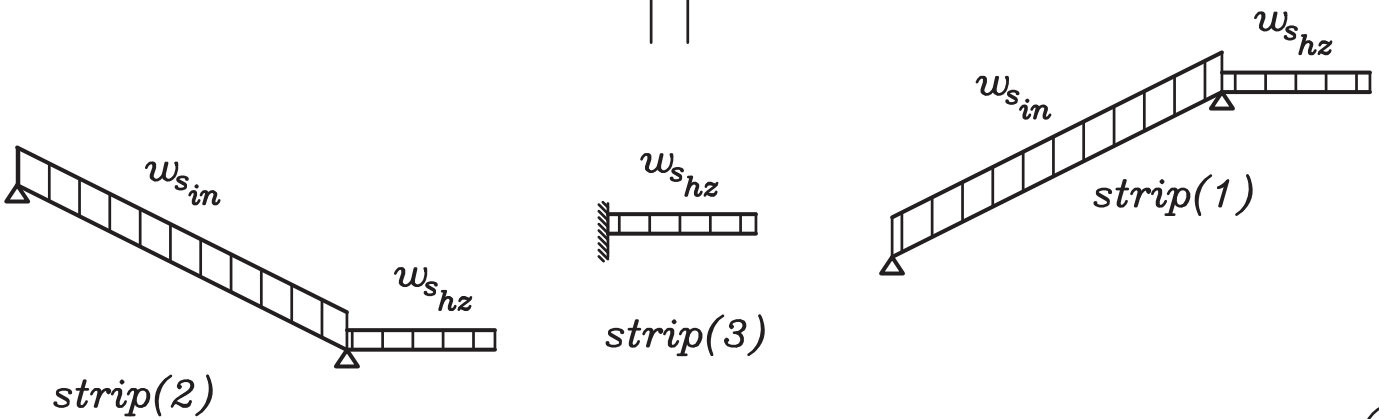
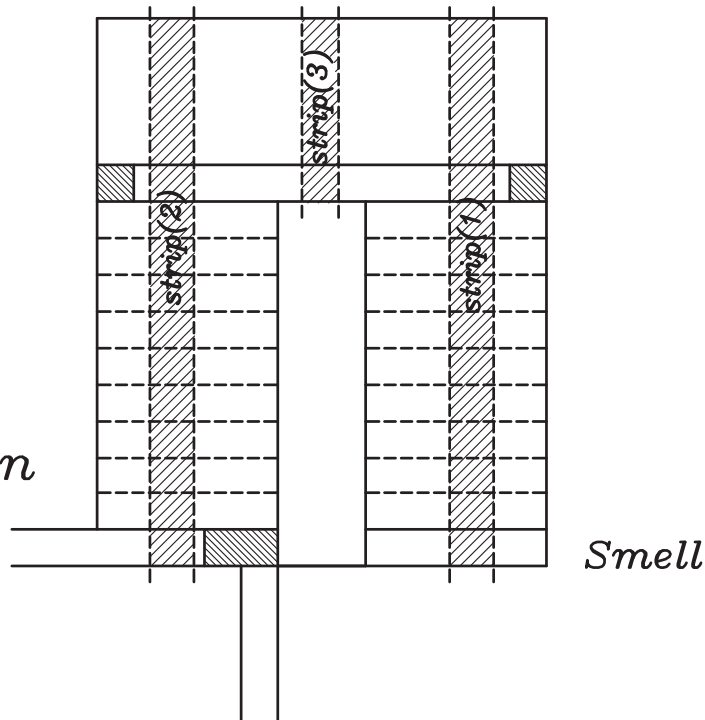


Example 2: – Show How to solve this stair

Arc. Plan

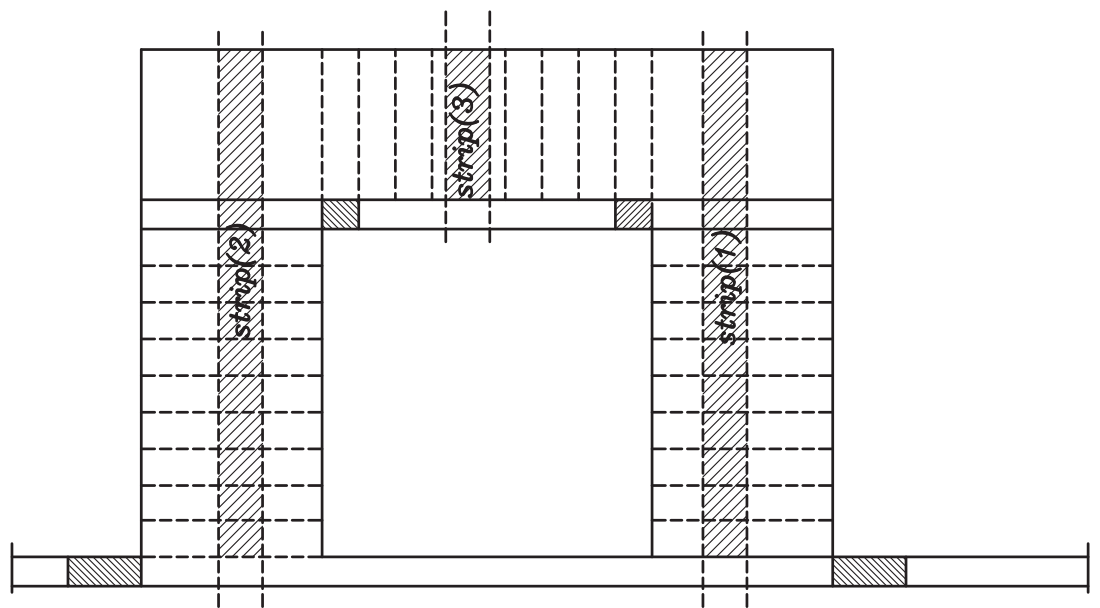
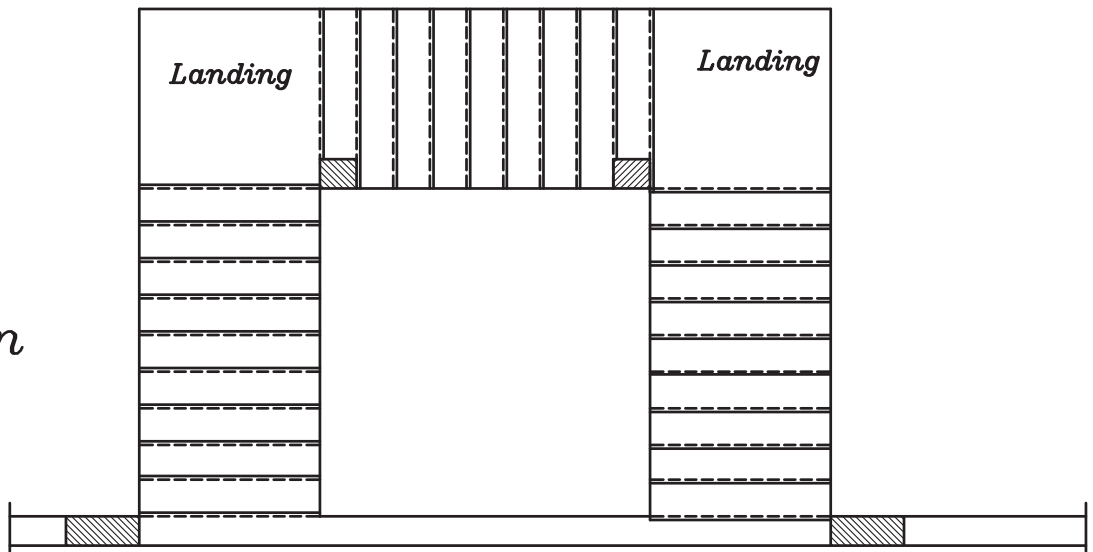


Structural Plan

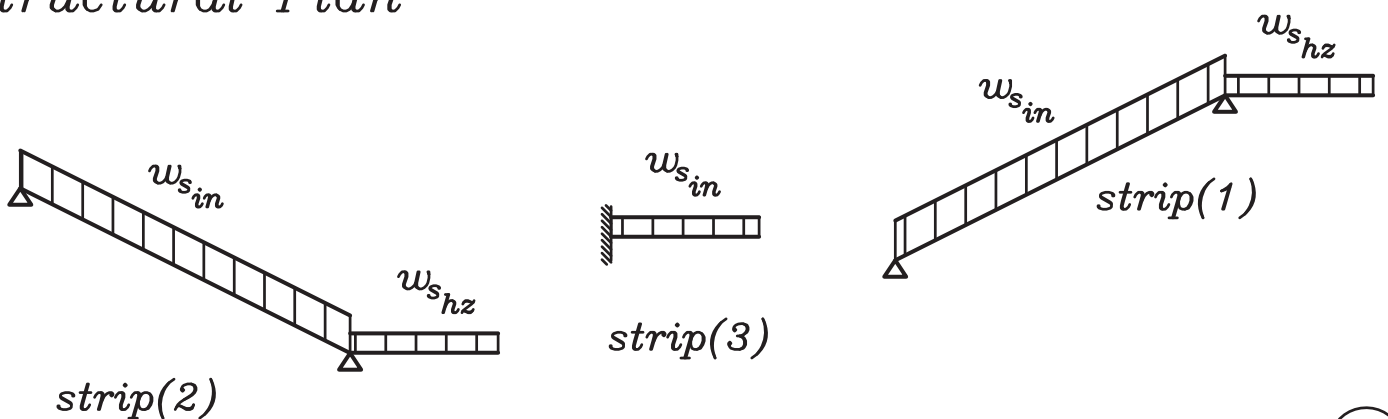


Example 3: – Show How to solve this stair

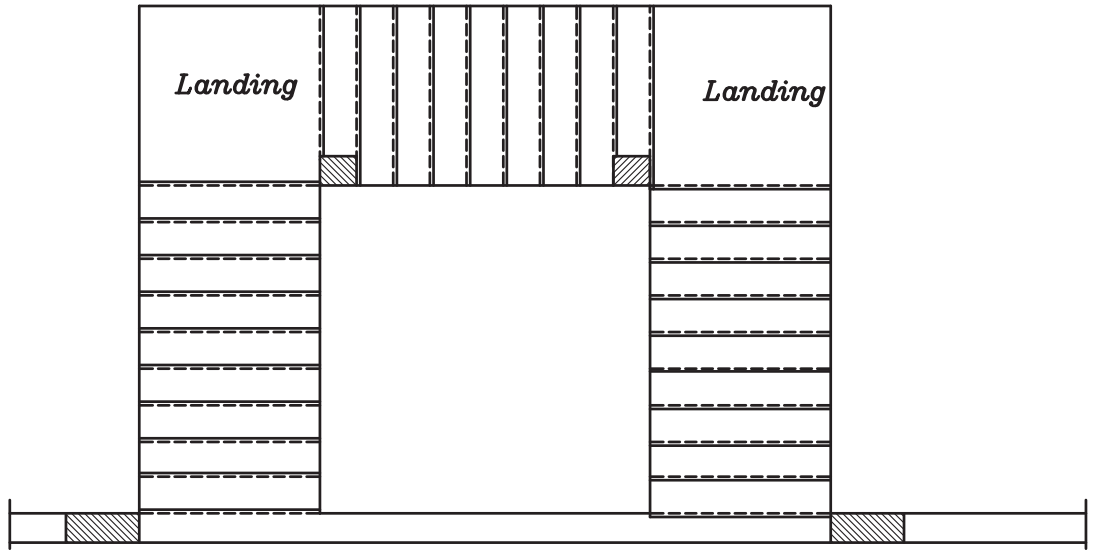
Arc. Plan



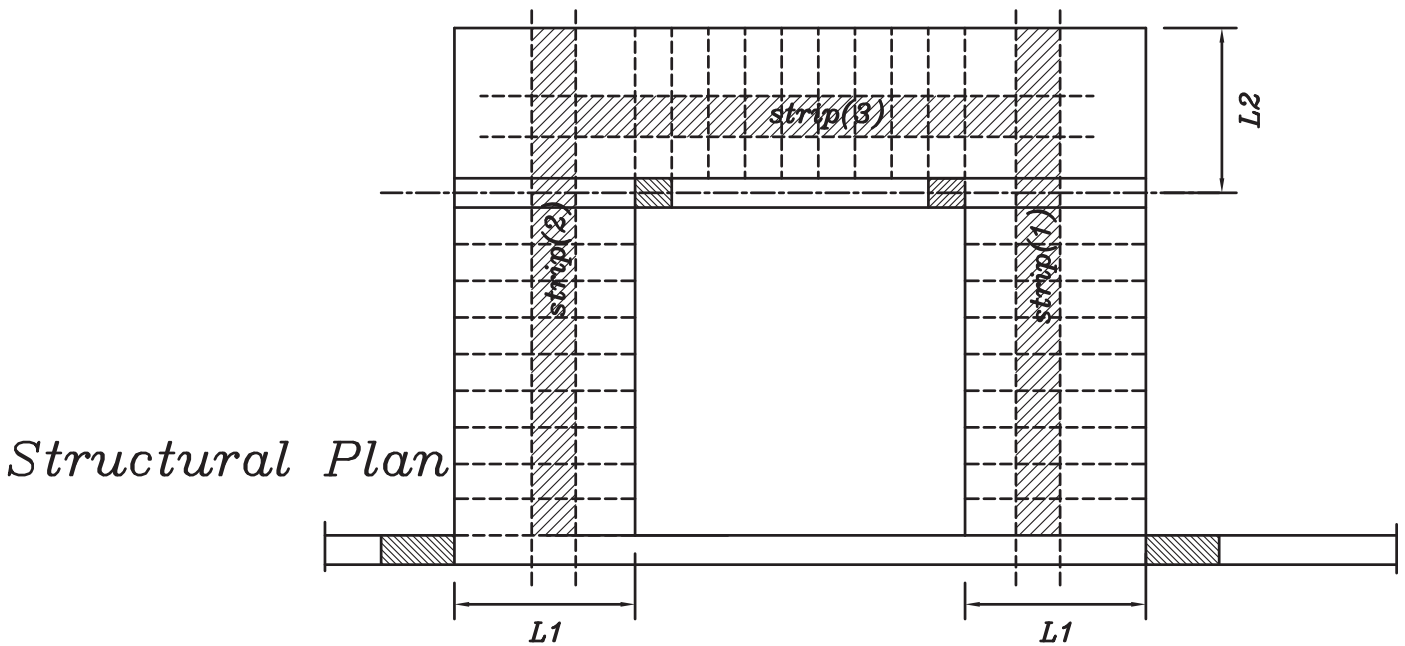
Structural Plan



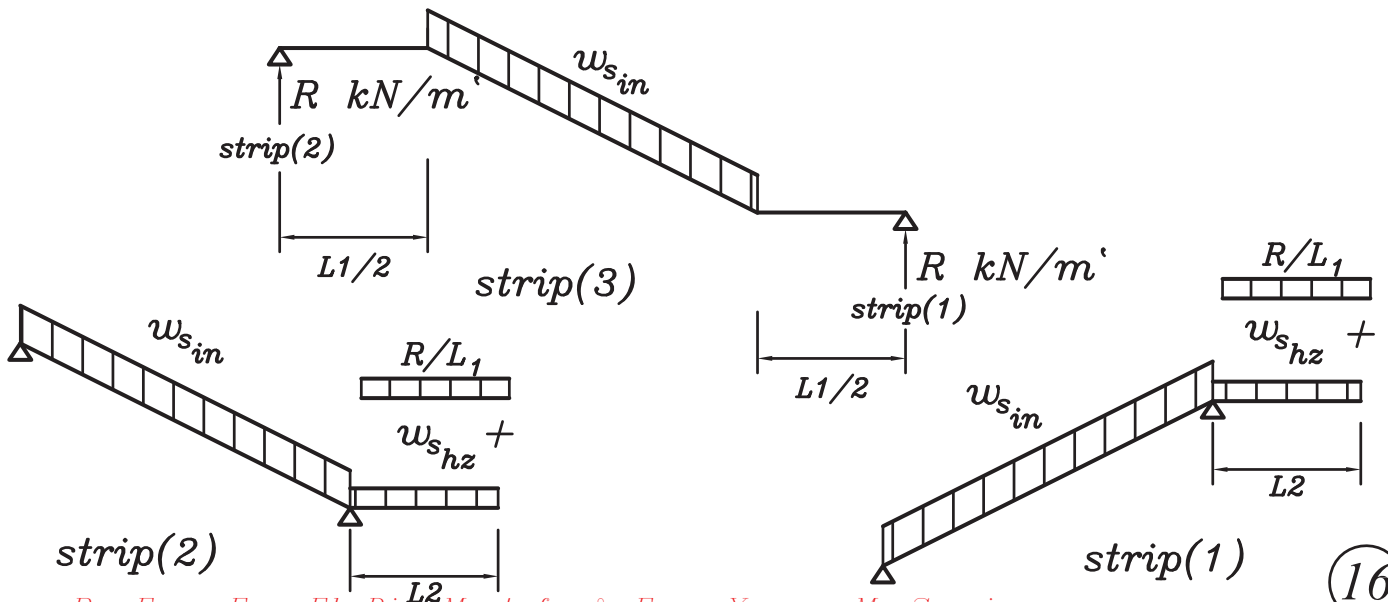
Another Solution



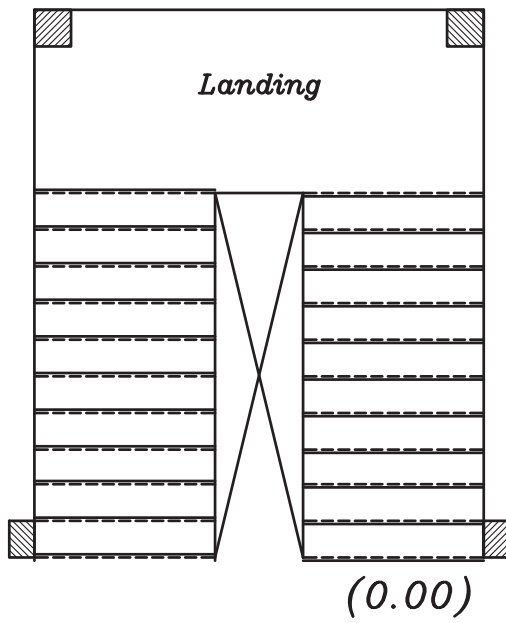
Arc. Plan



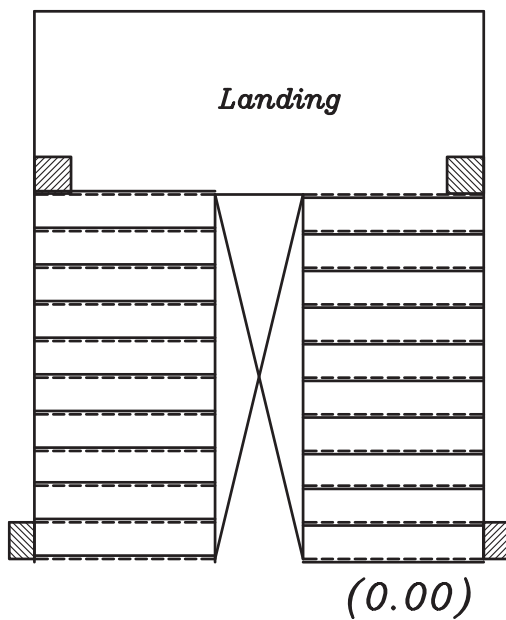
Structural Plan



Example 4: – Show How to solve this stairs

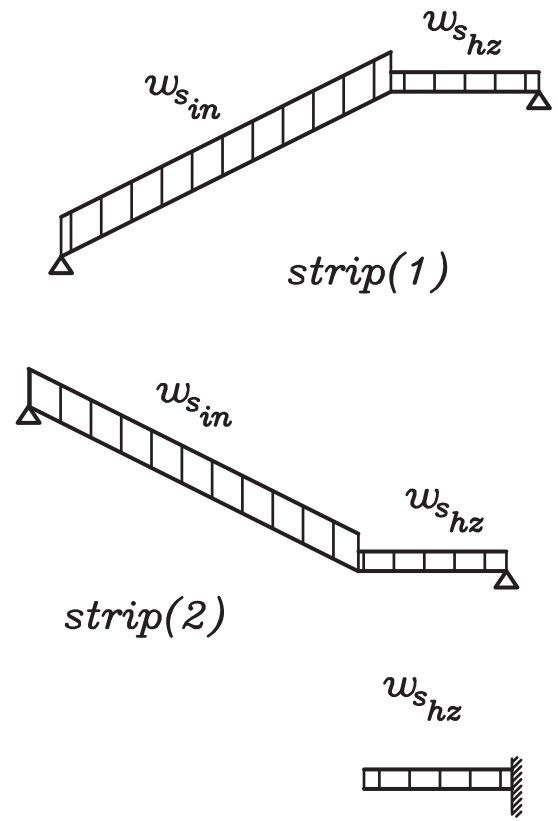
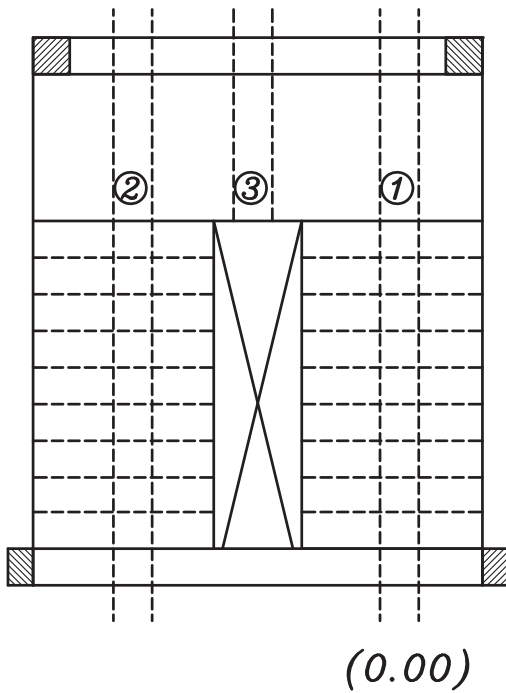


Case 1: –

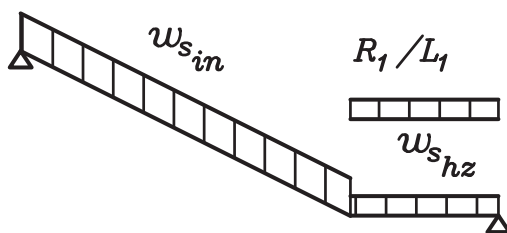
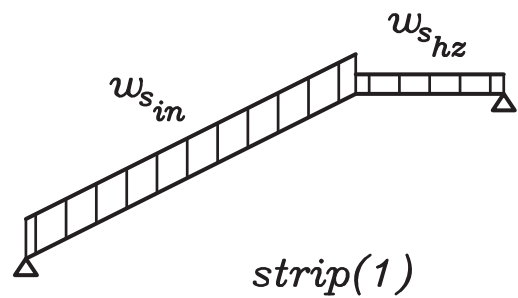
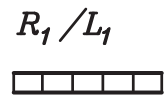
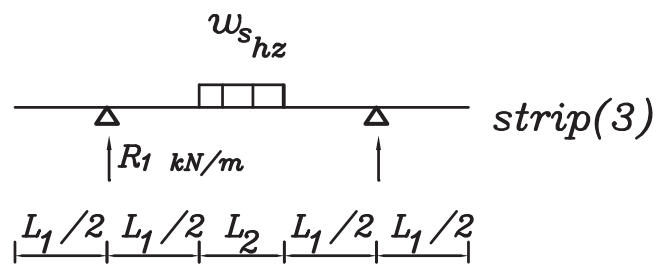
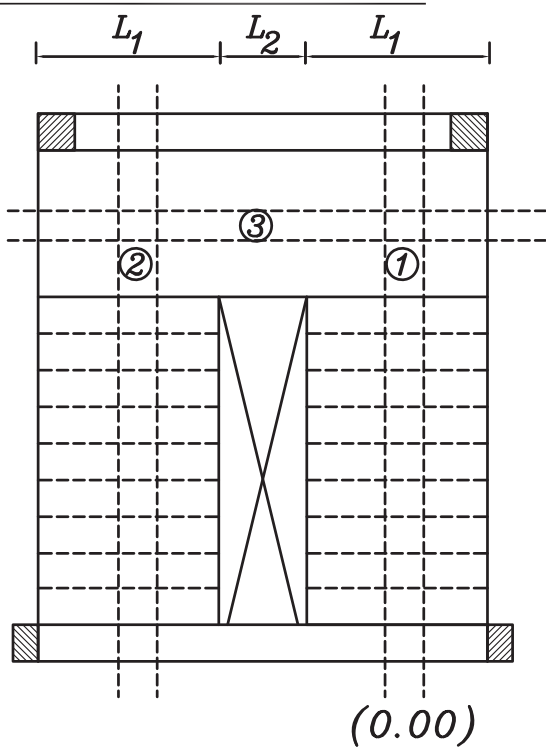


Case 2: –

Case 1:–

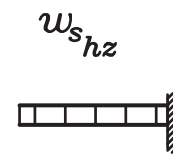
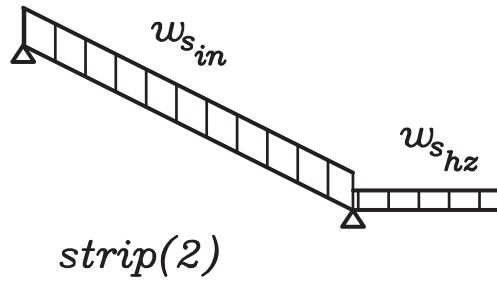
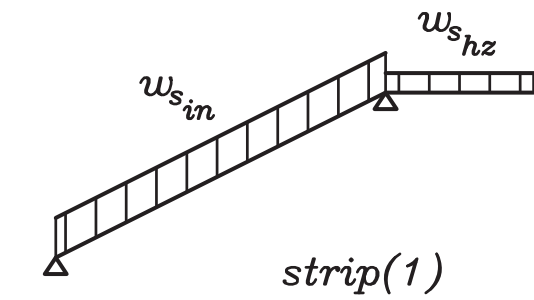
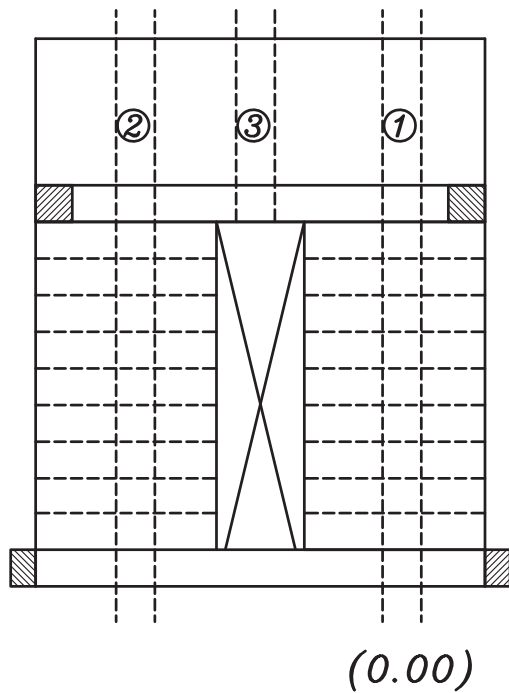


Alternative solution

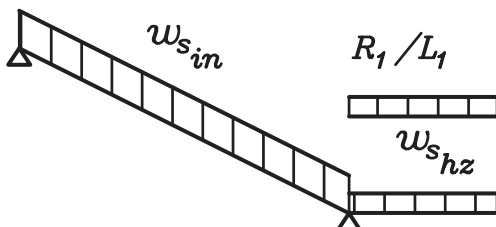
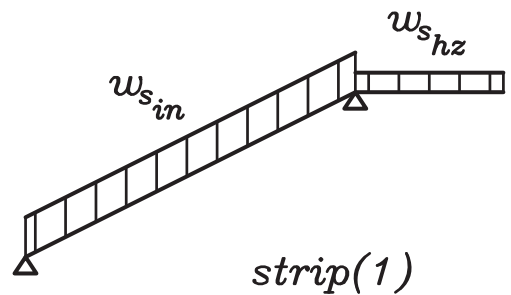
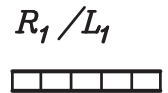
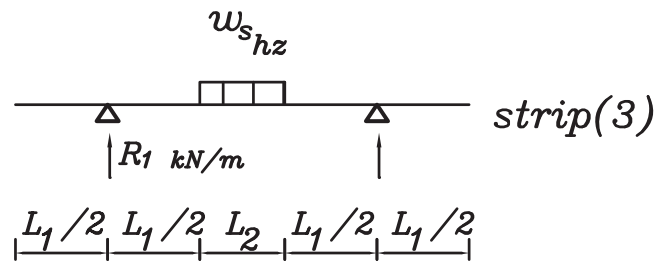
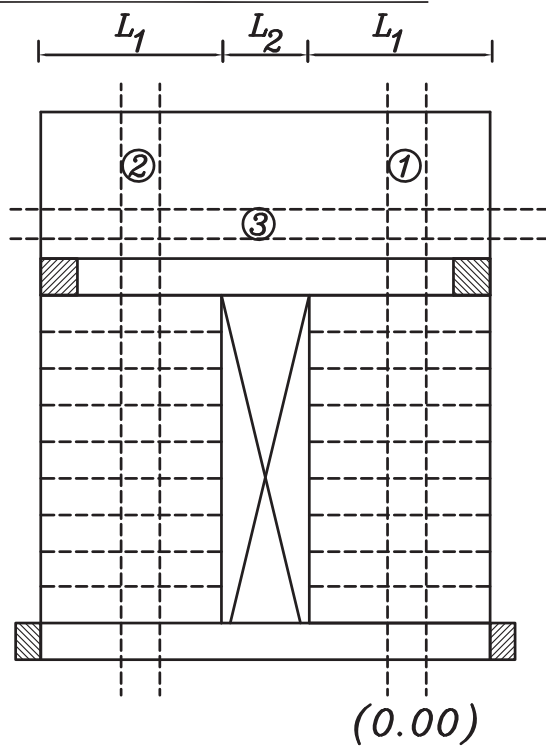


strip(2)

Case 2: -

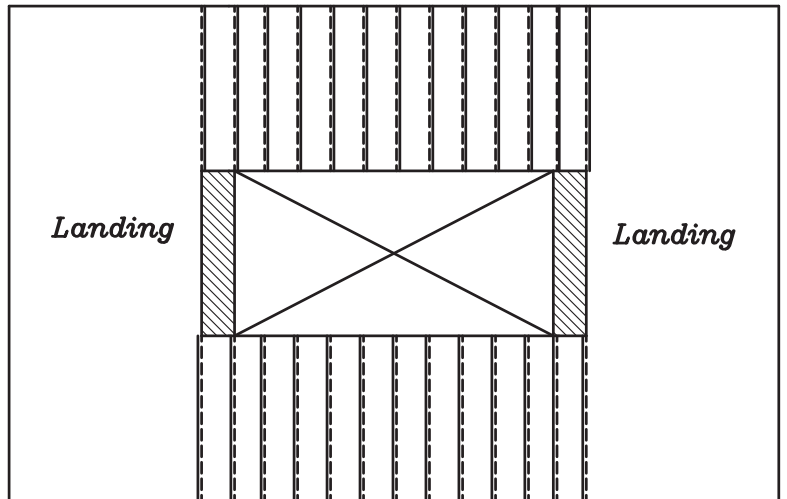


Alternative solution

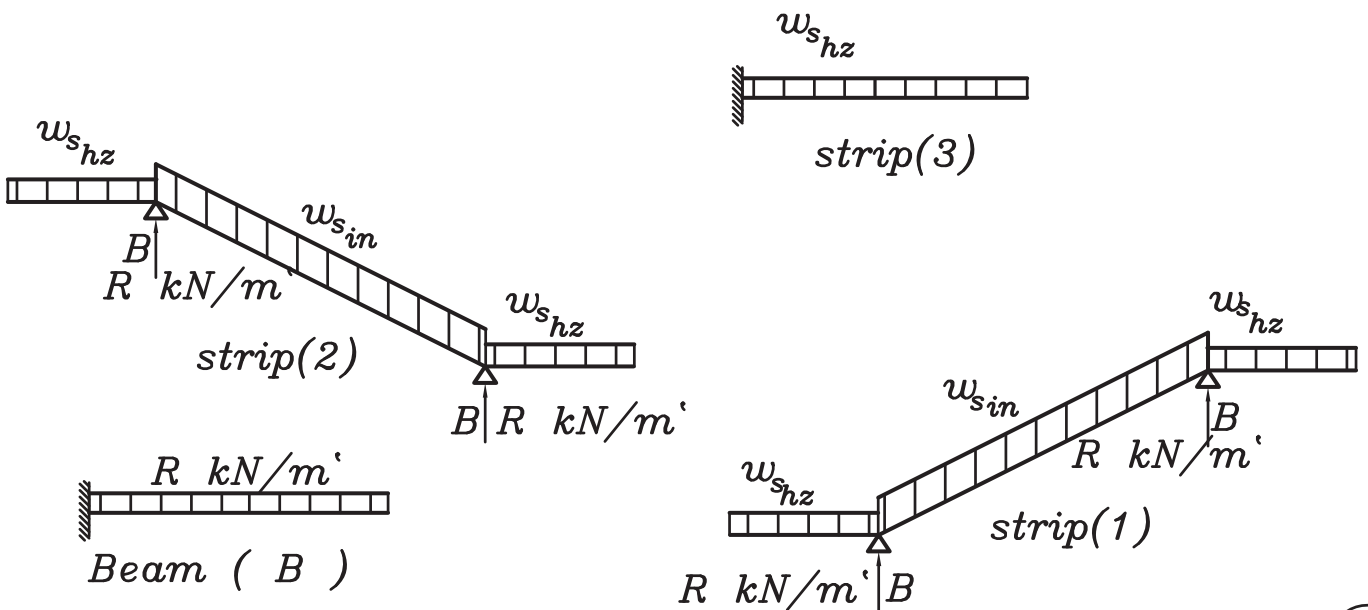
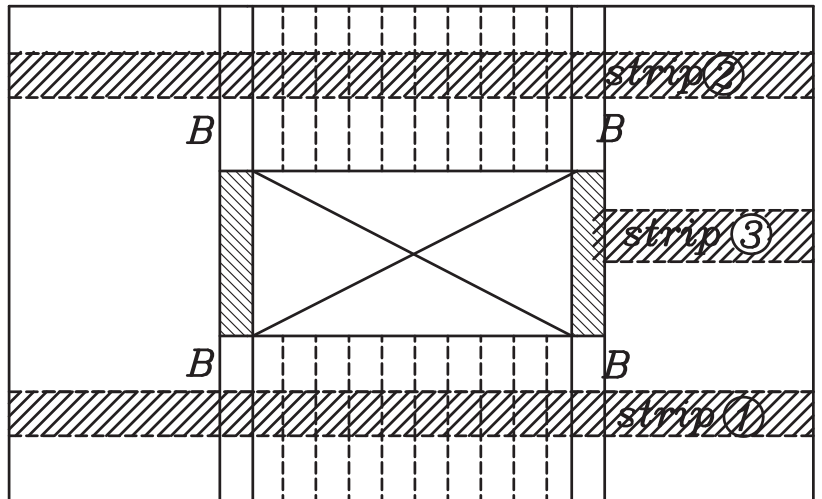


Example 5:– Show How to solve this stair

Arc. Plan

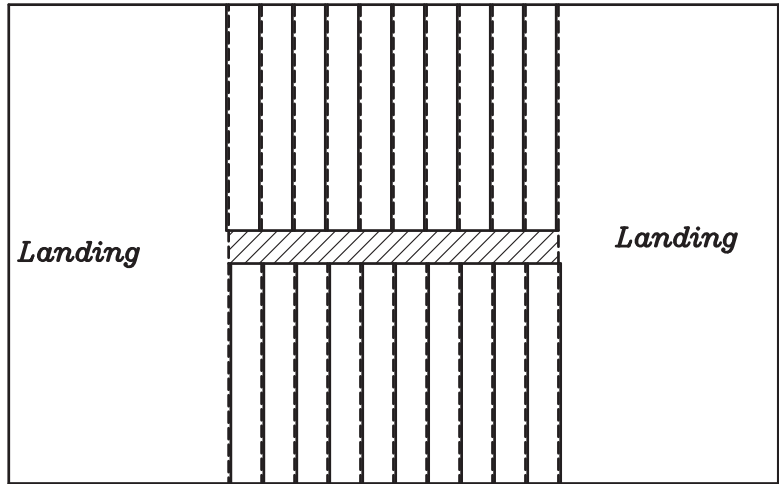


Structural Plan

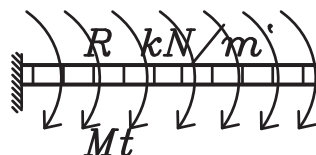
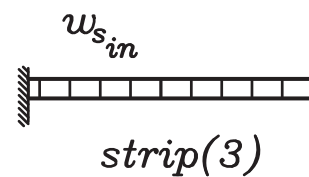
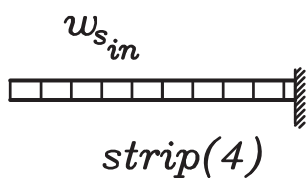
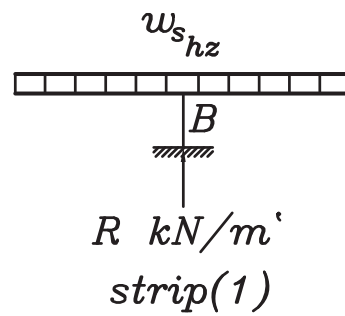
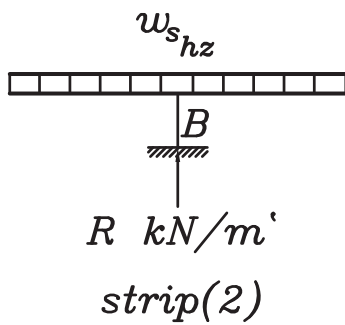
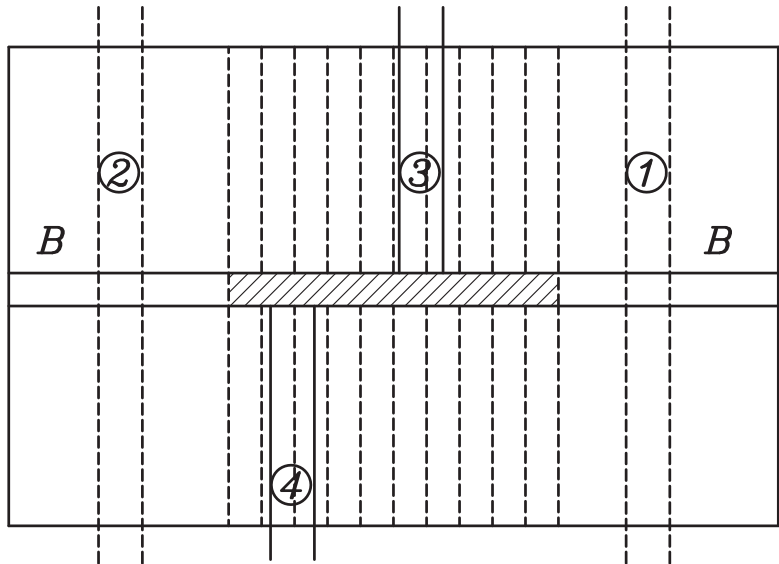


Example 6: – Show How to solve this stair

Arc. Plan



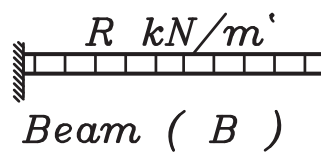
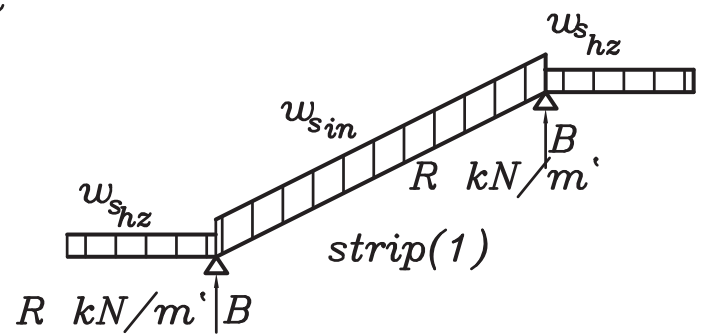
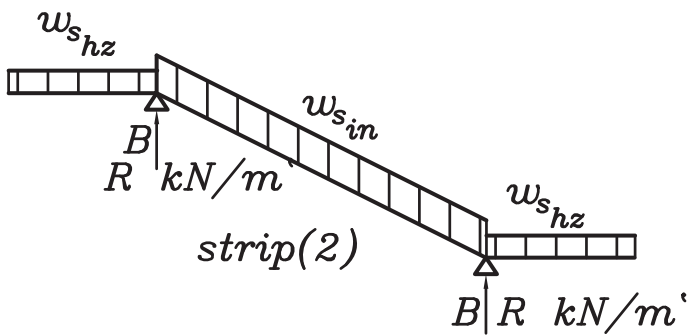
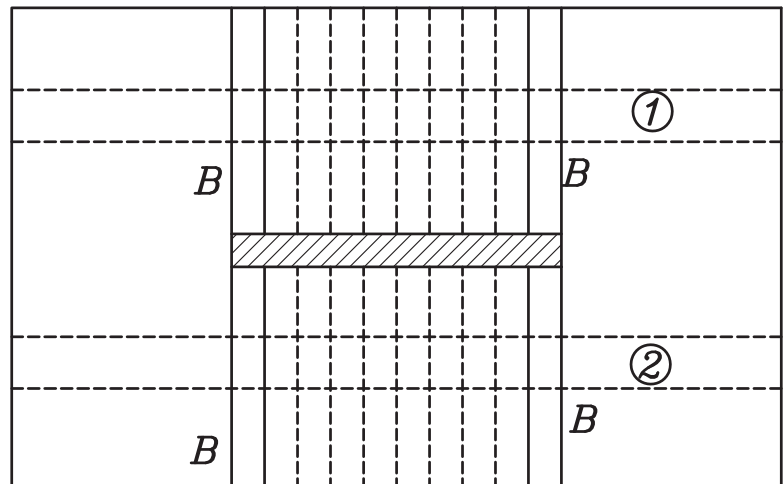
Structural Plan



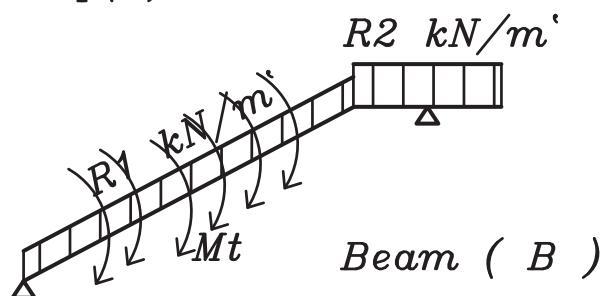
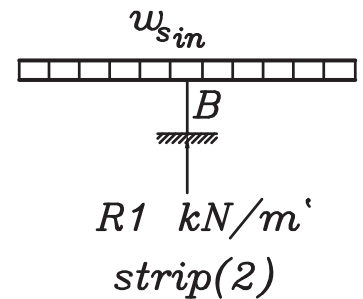
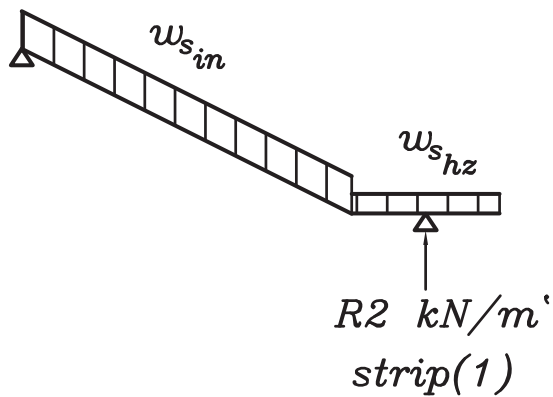
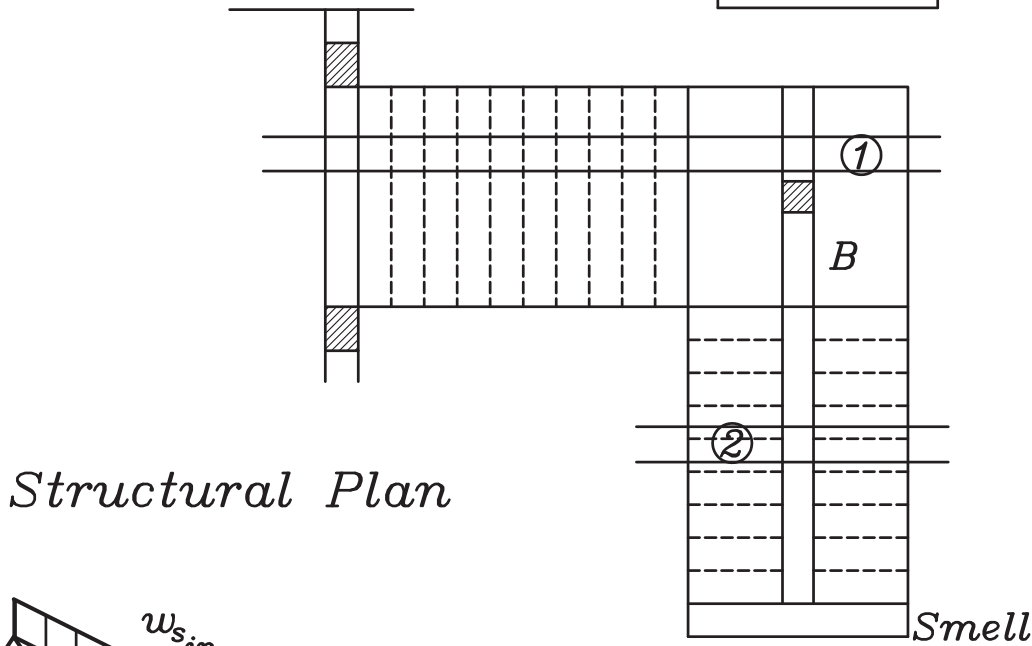
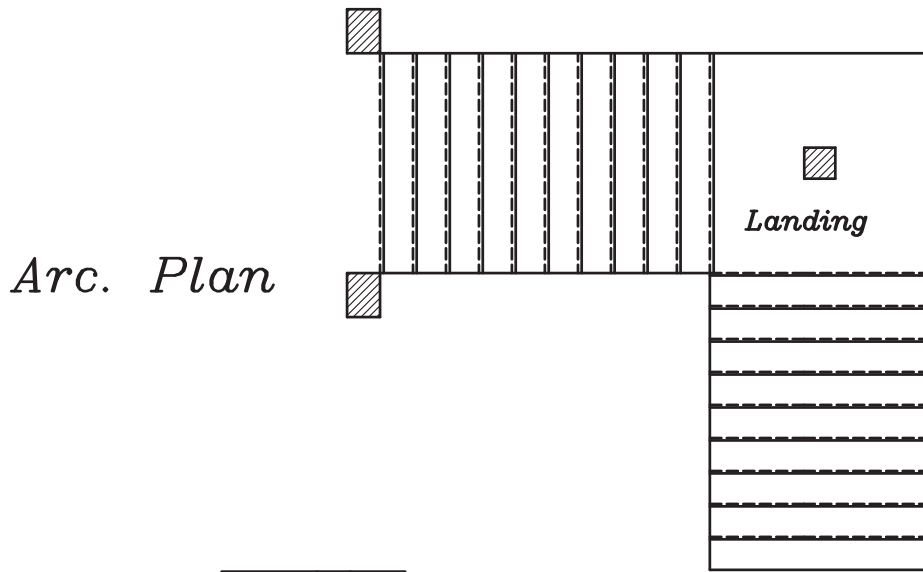
Beam (B)

Another Solution

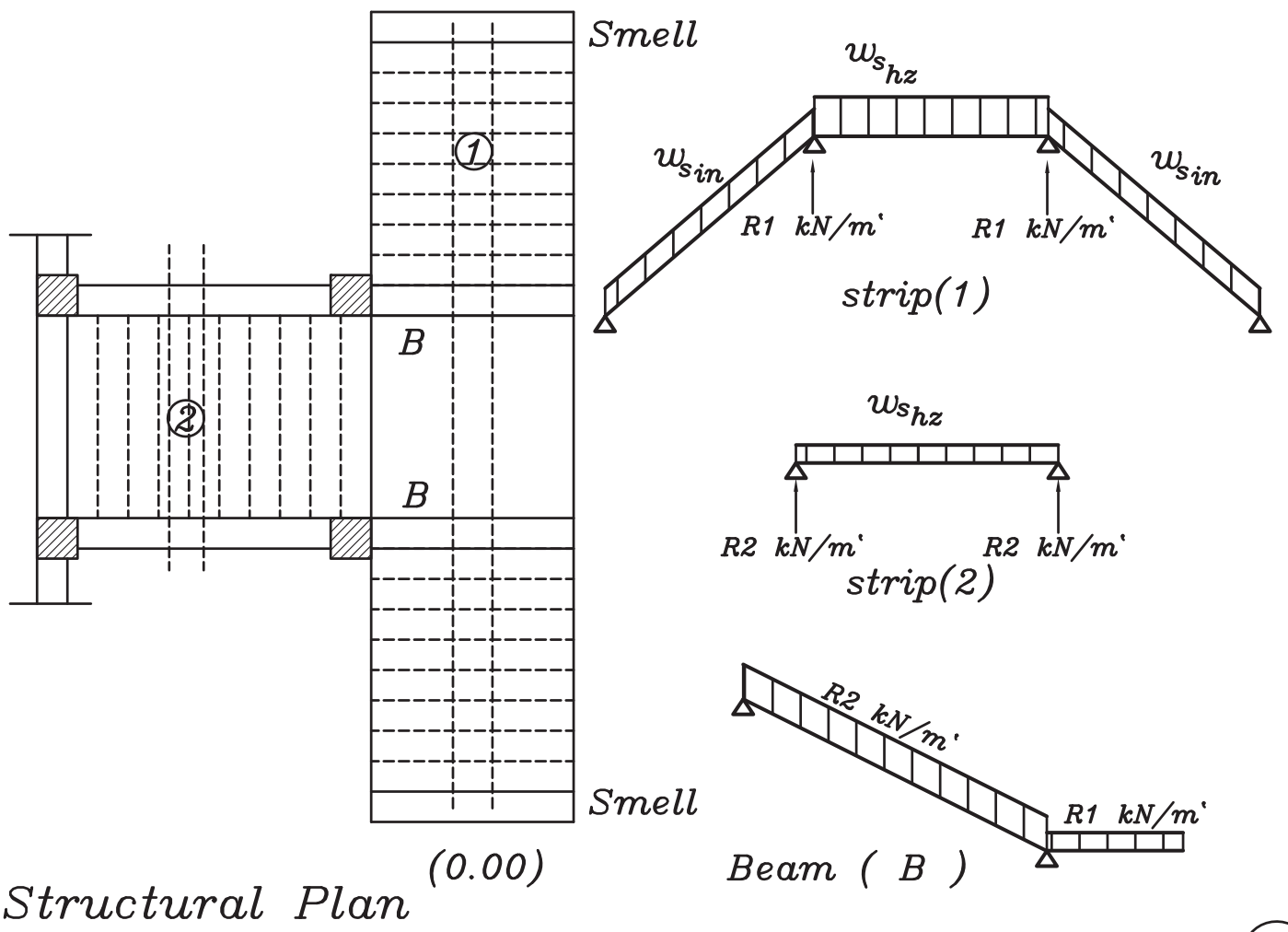
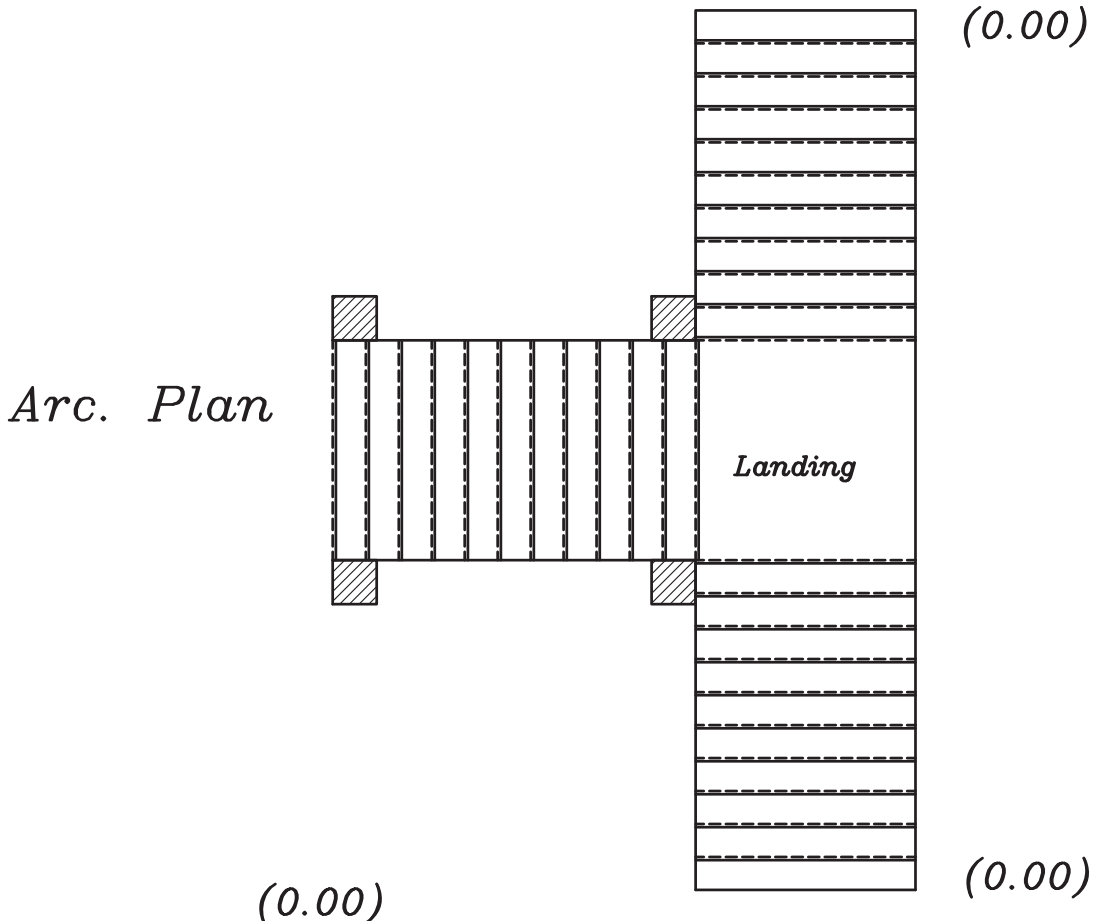
Structural Plan



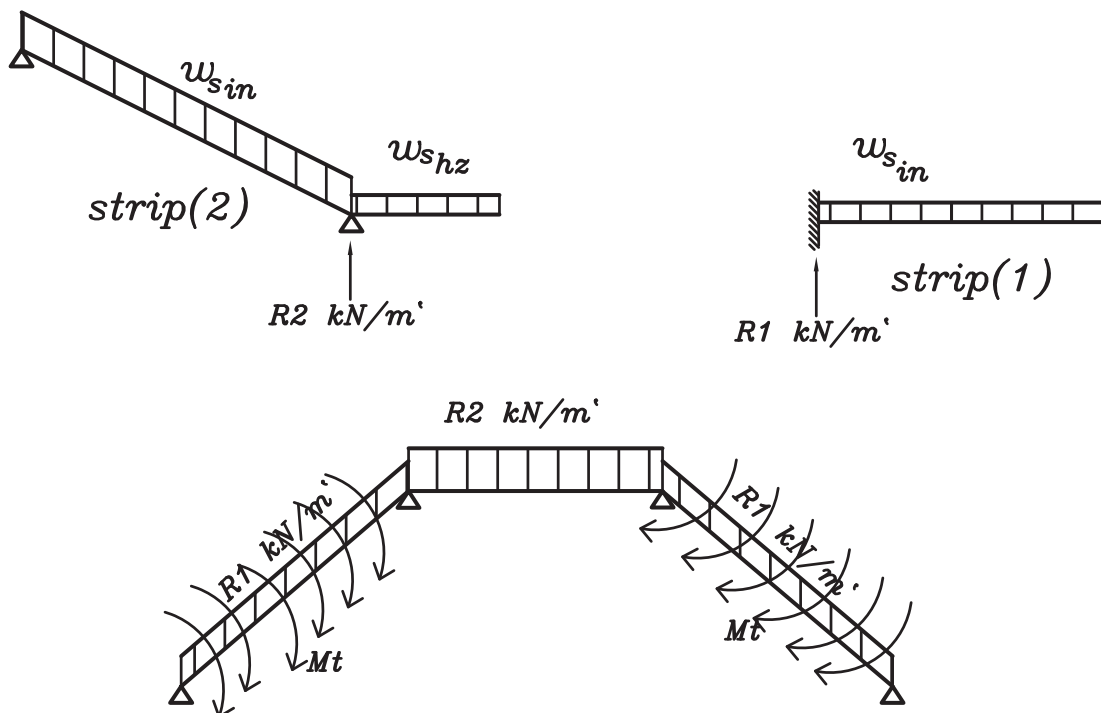
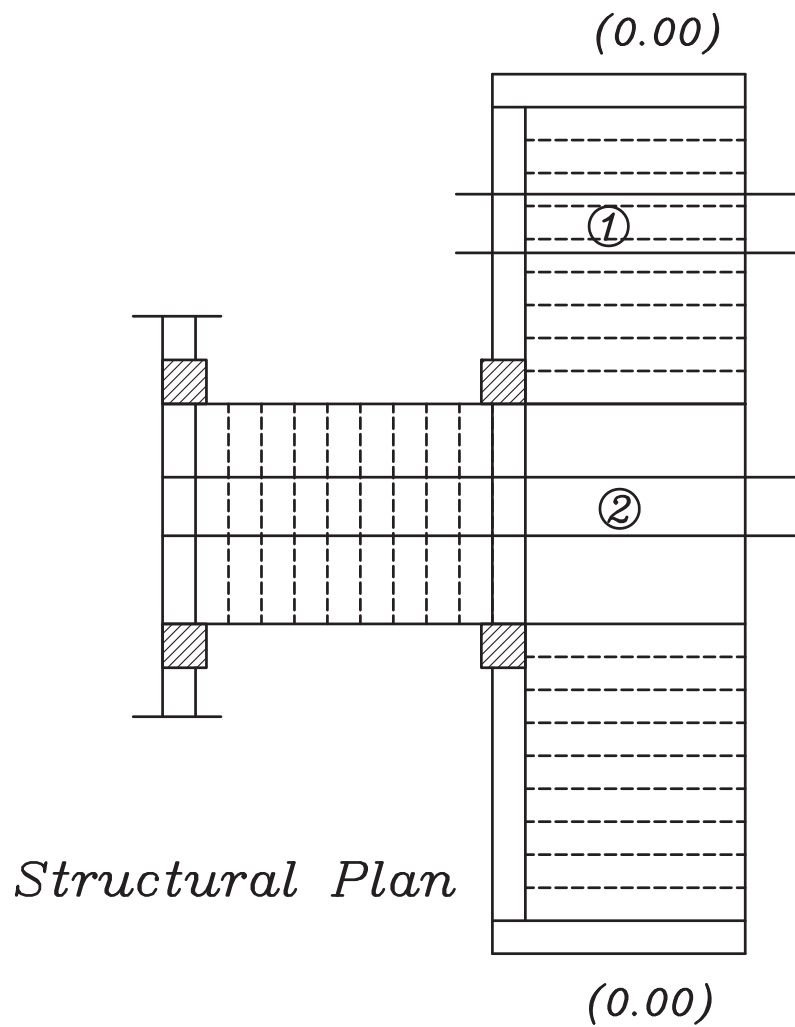
Example 7:– Show How to solve this stair



Example 8: – Show How to solve this stair



Another solution



Example 9: –

The following figure show the general layout of stair cases (each step 300x150), proceed with the following :

- 1- Complete design including all slabs and their supporting beams
- 2- Draw to a convenient scale the details of reinforcement of the stair and the supporting beams in plan & sections.

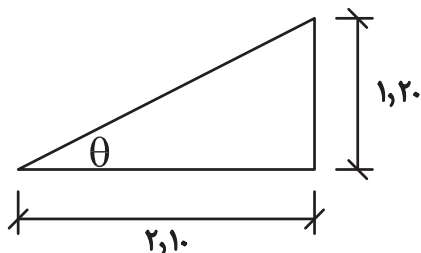
$$F.C. = 1.50 \text{ kN/m}^2$$

$$L.L. = 3.00 \text{ kN/m}^2$$

$$f_{cu} = 25 \text{ N/mm}^2$$

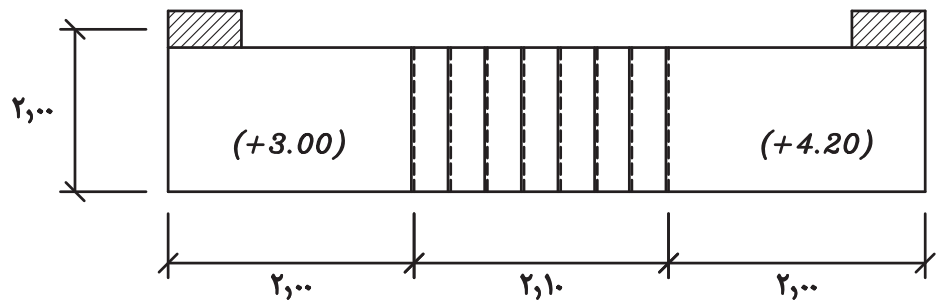
Steel used 360/520

$$O.w \text{ of beam} = 4.0 \text{ kN/m}$$

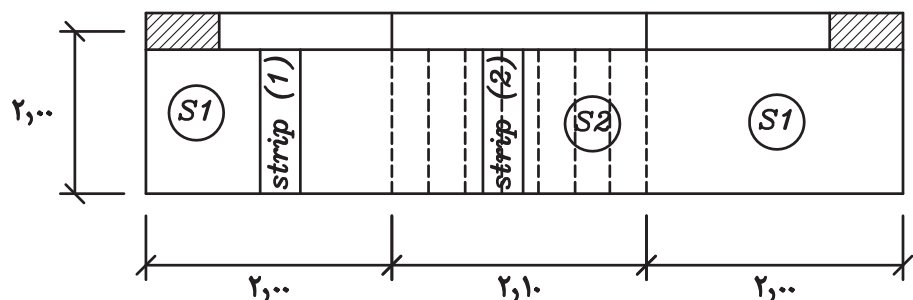


$$\tan \theta = \frac{1.20}{2.10} = 0.571$$

$$\theta = 29.75$$



Solution



1-slab thickness

for Cantilever slab S1

$$t_s = \frac{L_c}{15} + 20 = \frac{2000}{15} + 20 = 153.33 \text{ mm}$$

for Cantilever slab S2

$$t_s = \frac{L_c}{15} + 20 = \frac{2000}{15} + 20 = 153.33 \text{ mm}$$

⇒ Take $t_s = 16 \text{ cm}$ (check deflection)

$$t_{av} = t_s + 7 = 23 \text{ cm}$$

2-Calculation of load

For Landing :-

$$w_{shz} = 1.4[t_s \gamma_c + F.c.] + 1.6L.L. \quad kN/m^2$$

$$w_{shz} = 1.4[0.16*25 + 1.50] + 1.6*3.00$$

$$w_{shz} = 12.50 \quad kN/m^2$$

For Flight :-

$$w_{sin} = 1.4[t_{sav} \gamma_c + F.c.] + 1.6L.L \cos \theta \quad kN/m^2$$

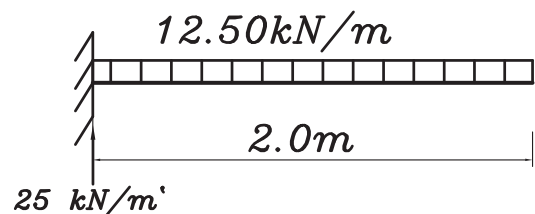
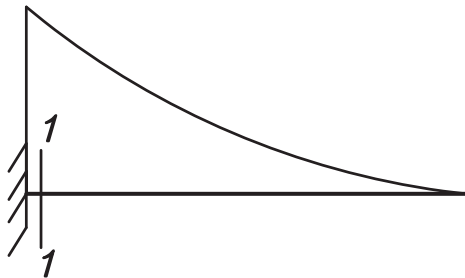
$$w_{sin} = 1.4[0.23*25 + 1.50] + 1.6*3.00 * \cos(29.75)$$

$$w_{sin} = 14.32 \quad kN/m^2$$

-Design of sections

Strip (1) :-

25kN.m



Sec(1-1)

$$M_{u.l.} = 25.00 \text{ kN.m} \quad \& \quad B = 1000 \text{ mm} \quad \& \quad d = 160 - 20 = 140 \text{ mm}$$

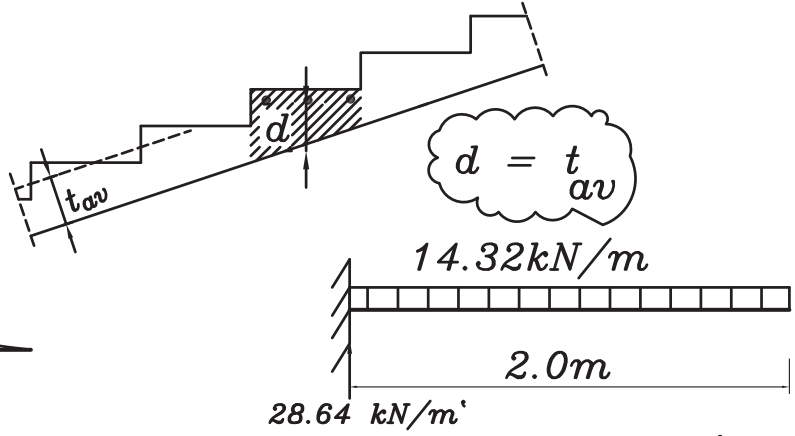
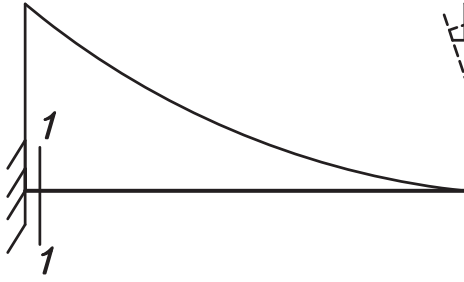
$$140 = C_1 \sqrt{\frac{25.00 * 10^6}{1000 * 25}} \quad C_1 = 4.43 \quad \& \quad J = 0.813$$

$$A_s = \frac{25.00 * 10^6}{0.813 * 140 * 360} = 610.13 \quad \text{mm}^2/\text{m}'$$

$$A_s = \underline{5 \phi 13/\text{m}'}$$

Strip (2) :-

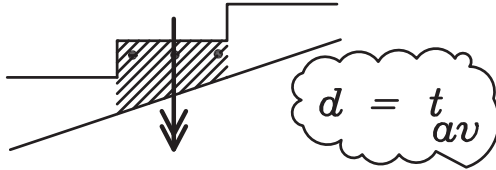
28.64kN.m



- ملحوظة

لاحظ أننا لا نعتبر هذه الشريحة شريحة أفقية في بلاطة مائلة لأن العزم سالب وعمودى على اتجاه الحديد لذلك فهو لا يحتاج الى تحليل

Sec(1-1)



$M_{u.l.} = 28.64 \text{ kN.m}$ & $B = 1000 \text{ mm}$ & $d = 230 \text{ mm}$

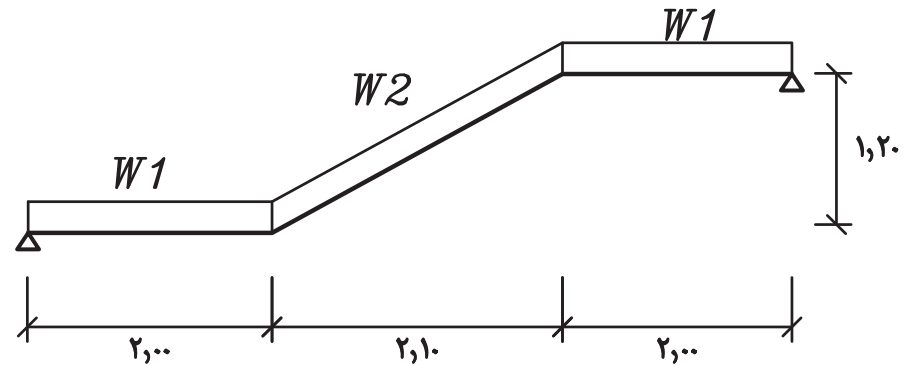
$230 = C_1 \sqrt{\frac{28.64 * 10^6}{1000 * 25}}$ $C_1 = 6.8$ & $J = 0.826$

$A_s = \frac{28.64 * 10^6}{0.826 * 230 * 360} = 418.76 \text{ mm}^2/\text{m}'$

$A_s / \text{step} = 418.76 * 0.3 = 125.63 \text{ mm}^2/\text{step}$

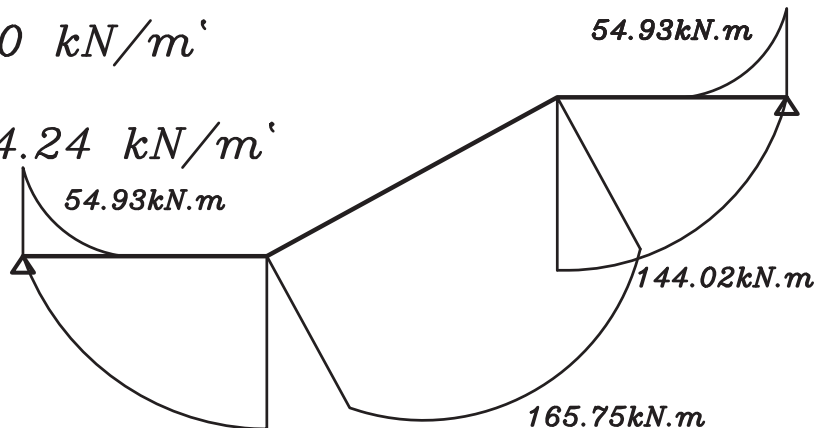
$A_s = 2 \phi 10 / \text{step}$

-Design of Beam



$W1 = 4 * 1.4 + 25 = 30.60 \text{ kN/m}'$

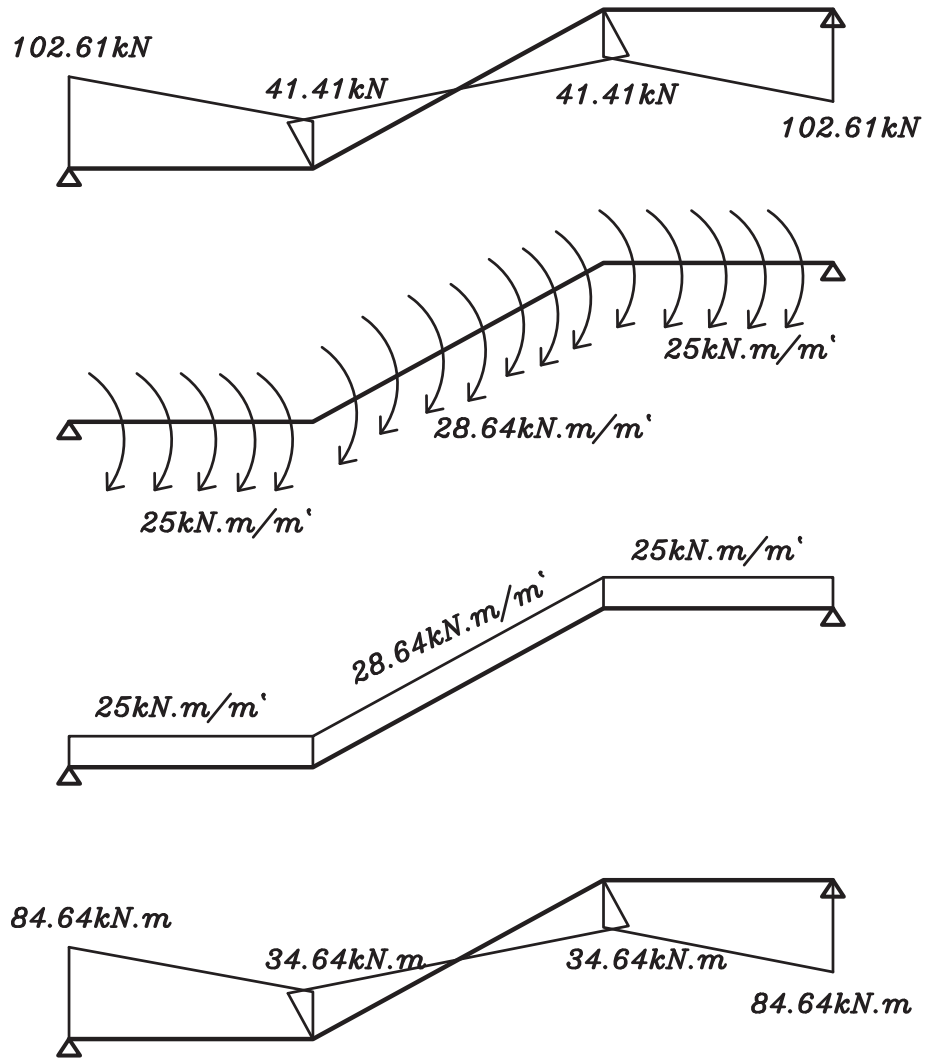
$W2 = 4 * 1.4 + 28.64 = 34.24 \text{ kN/m}'$



لاص

144.02kN.m

سببی



Take $b = 400 \text{ mm}$

$$1\text{-from bending} \Rightarrow d = 3.5 \sqrt{\frac{165.75 \cdot 10^6}{400 \cdot 25}} = 451.00 \text{ mm}$$

$$d = 500 \text{ mm} \quad t = d + 50 \text{ mm} = \underline{550 \text{ mm}}$$

$$2\text{-from Torsion} \Rightarrow \frac{3M_{tu}}{b^2 t} = 2 \text{ N/mm}^2 \Rightarrow \frac{3 \cdot 84.64 \cdot 10^6}{400^2 \cdot t} = 2.00$$

$$\underline{t = 793.5 \text{ mm}} \Rightarrow \text{Take } t = 800 \text{ mm}$$

Take Beam = (400 x 800)

By Eng. Ezz El-Din Mostafa & Eng. Yasser M. Samir

Design for shear+Torsion:

$$q_{su} = \frac{Q_u}{bd} = \frac{102.61 \cdot 10^3}{400 \cdot 750}$$

$$q_{su} = 0.342 \text{ N/mm}^2$$

$$q_{tu} = \frac{M_{tu}}{1.7 A_{oh} t_e}$$

$$A_{oh} = (400 - 50)(800 - 50)$$

$$A_{oh} = 262500 \text{ mm}^2$$

$$P_h = 2[350 + 750] = 2200 \text{ mm}$$

$$t_e = \frac{A_{oh}}{P_h} = 119.32 \text{ mm}$$

$$q_{tu} = \frac{84.64 \cdot 10^6}{1.7 \cdot 262500 \cdot 119.32}$$

$$q_{tu} = 1.59 \text{ N/mm}^2$$

$$q_{cu} = 0.24 \sqrt{25/1.5} = 0.98 \text{ N/mm}^2 \quad q_{tu_{min}} = 0.06 \sqrt{25/1.5} = 0.24 \text{ N/mm}^2$$

$$q_{u_{max}} = 0.70 \sqrt{25/1.5} = 2.86 \text{ N/mm}^2$$

$$\sqrt{q_{su}^2 + q_{tu}^2} = \sqrt{(0.34)^2 + (1.59)^2} = 1.63 \text{ N/mm}^2 < q_{u_{max}} \quad (\text{ok})$$

(Design for Torsion)

$$A_{str} = \frac{M_{tu} \cdot S}{1.7 A_{oh} \left(\frac{f_{yst}}{\gamma_s} \right)}$$

$$\Rightarrow \frac{A_{str}}{S} = \frac{84.64 \cdot 10^6}{1.7 \cdot 262500 \cdot 360 / 1.15}$$

$$\frac{A_{str}}{S} = 0.606$$

assume $A_{str} = \phi 10 = 78.5 \text{ mm}^2$

$$\frac{78.5}{S} = 0.606 \Rightarrow S = 129.54 \text{ mm} > 100 \text{ mm}$$

$$\text{No. of stirrups/m} = \frac{1000}{S} = 7.72$$

Take stirrups $8 \phi 10/\text{m}$ (2 branches)

$$A_{sL} = \left(\frac{A_{str}}{S} \right) \cdot (P_h) \cdot \left(\frac{f_{yst}}{f_{y.l.b.}} \right)$$

$$A_{sL} = 0.606 \cdot 2200 \cdot \frac{360}{360} = 1333.2 \text{ mm}^2$$

$$\frac{A_{sL}}{4} = \frac{1333.2}{4} = 333.3 \text{ mm}^2$$

Design for B.M.

Sec(1-1) (L-sec)

$$B = \begin{cases} 6*0.16+0.4=1.36 \text{ m} \\ \text{C.L.} \rightarrow \text{C.L.}=2.0+0.2=2.20\text{m} \\ \frac{1*6.42}{10} + 0.40 = 1.04\text{m} \end{cases}$$

$$B=1040\text{mm} \ \& \ d=800-50\text{mm}=750\text{mm} \ \& \ M_u=165.75 \text{ kN.m}$$

$$750=C_1 \sqrt{\frac{165.75*10^6}{1040*25}} \quad C_1=9.4 \quad \& \quad J=0.826$$

$$A_s = \frac{165.75*10^6}{0.826*750*360} = 743.21\text{mm}^2$$

$$A_{s_{min}} = \frac{1.1}{f_y} bd = \frac{1.1}{360} 400*750 = 916.67\text{mm}^2 > A_s$$

$$A_{s_{min}} = \begin{cases} \frac{1.1}{f_y} bd = \frac{1.1}{360} 400*750 = 916.67\text{mm}^2 \\ 1.3A_{s_{req}} = 1.3*743.21 = 966.173\text{mm}^2 \\ \frac{0.15}{100} bd = \frac{0.15}{100} 400*750 = 450.00\text{mm}^2 \end{cases}$$

$$A_{s_{min}} = 916.67\text{mm}^2$$

$$\begin{aligned} A_{s_{total}} &= A_{s(B.M.)} + \frac{A_{sL}}{4} \\ &= 916.67 + 333.3 = 1250.0 \quad \text{mm}^2 \end{aligned}$$

$$A_{s_{total}} = \underline{5 \phi 18}$$

Sec(2-2) (R-sec)

$$b=400\text{mm} \ \& \ d=800-50\text{mm}=750\text{mm} \ \& \ M_u=54.93 \text{ kN.m}$$

$$750=C_1 \sqrt{\frac{54.93*10^6}{400*25}} \quad C_1=10.12 \quad \& \quad J=0.826$$

$$A_s = \frac{54.93*10^6}{0.826*750*360} = 246.30\text{mm}^2$$

$$A_{s_{min}} = \frac{1.1}{f_y} bd = \frac{1.1}{360} 400*750 = 916.67\text{mm}^2 > A_s$$

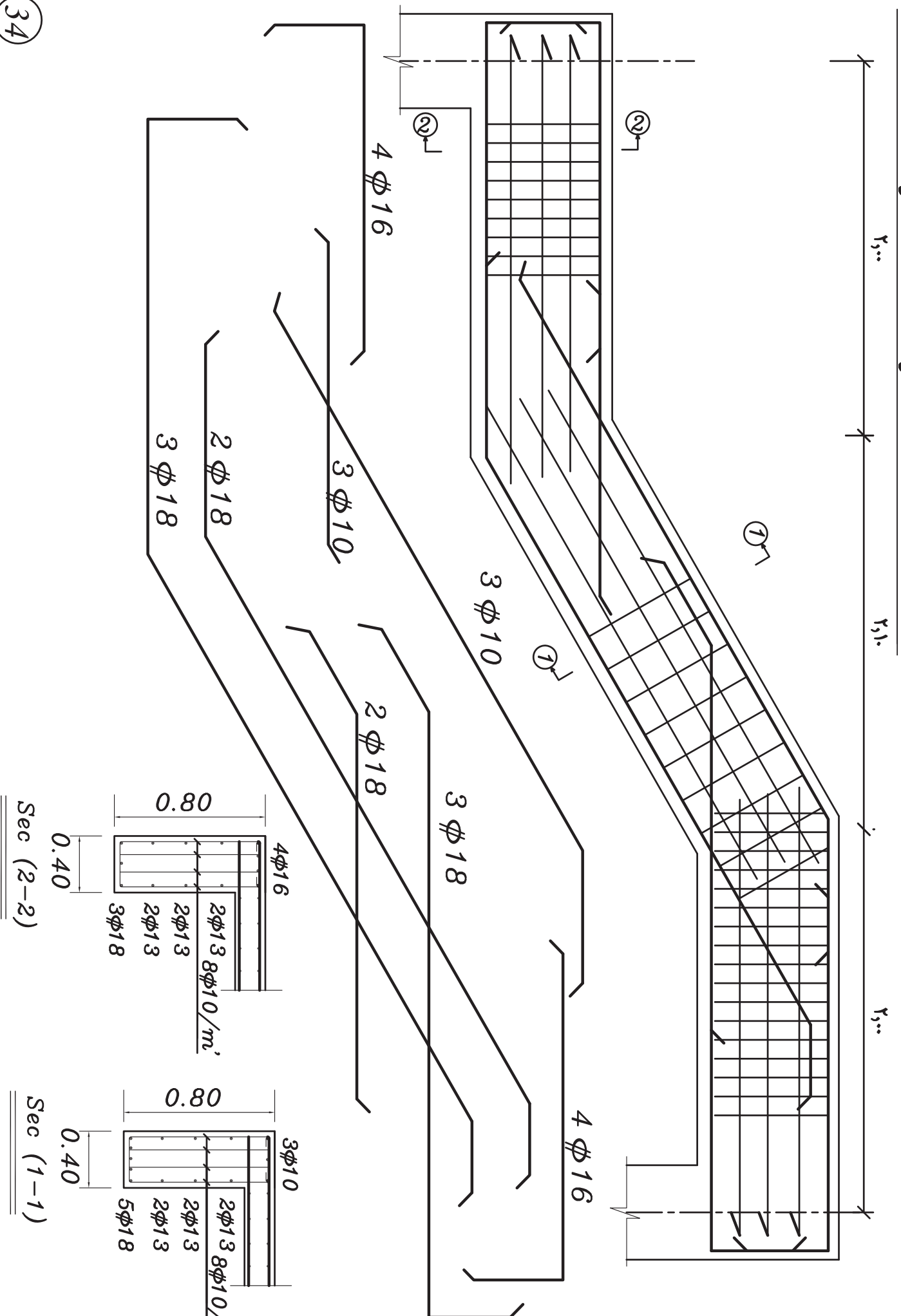
$$A_{s_{min}} = \left[\begin{array}{l} \frac{1.1}{f_y} bd = \frac{1.1}{360} 400*750 = 916.67\text{mm}^2 \\ 1.3A_{s_{req}} = 1.3*246.30 = 320.20 \text{ mm}^2 \\ \frac{0.15}{100} bd = \frac{0.15}{100} 400*750 = 450.00\text{mm}^2 \end{array} \right.$$

$$A_{s_{min}} = 450.00\text{mm}^2$$

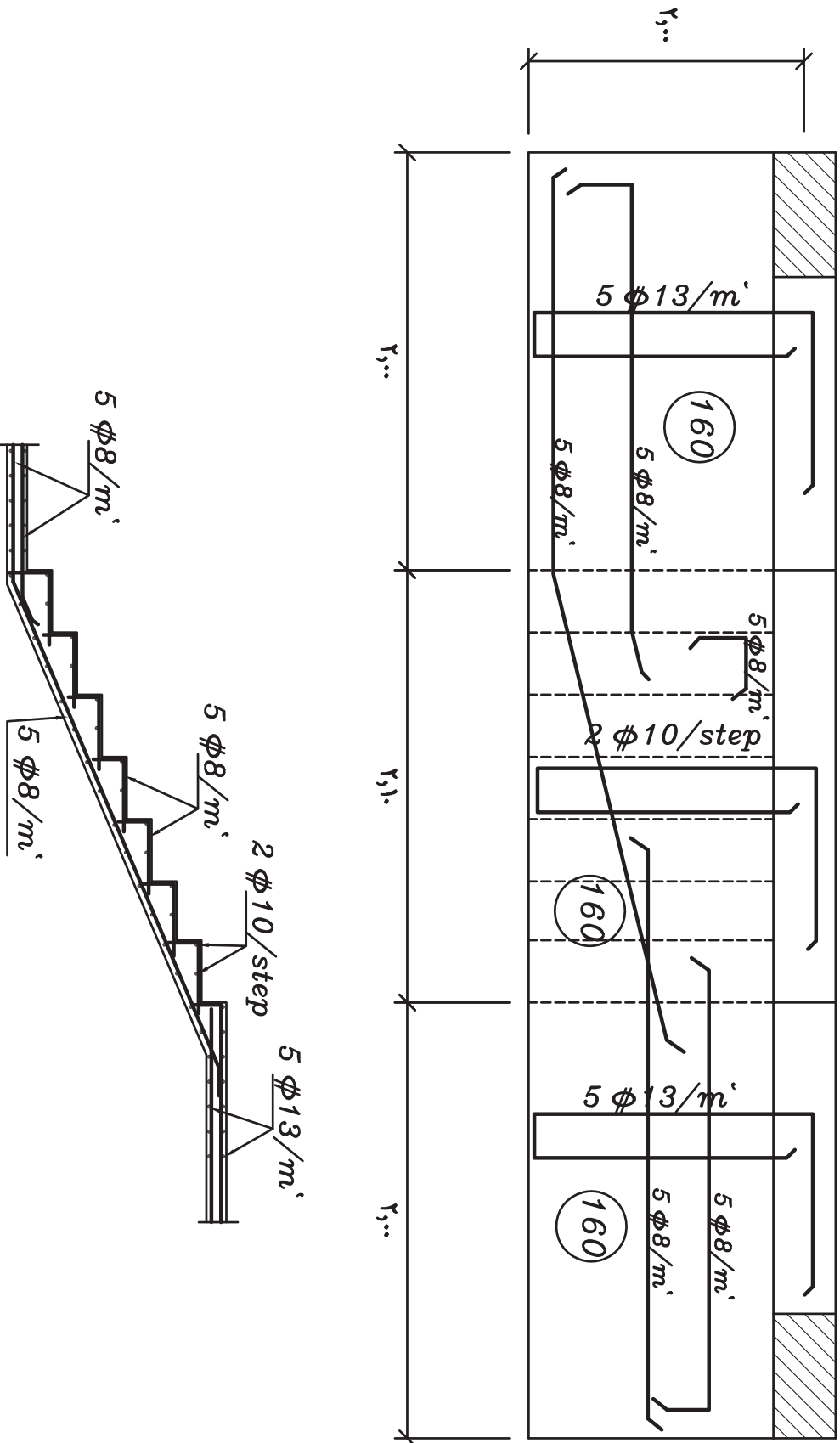
$$A_{s_{total}} = A_{s(B.M.)} + \frac{A_{sL}}{4} \\ = 450.00 + 333.3 = 783.30 \text{ mm}^2$$

$$A_{s_{total}} = \underline{4 \ \phi \ 16}$$

Details of R.F.T. for the beam



Details of R.F.T. for Stair



Example 10: –

The following figure show the general layout of stair cases (each step 300x150), proceed with the following :

- 1- Complete design including all slabs and their supporting beams
- 2- Draw to a convenient scale the details of reinforcement of the stair and the supporting beams in plan & sections.

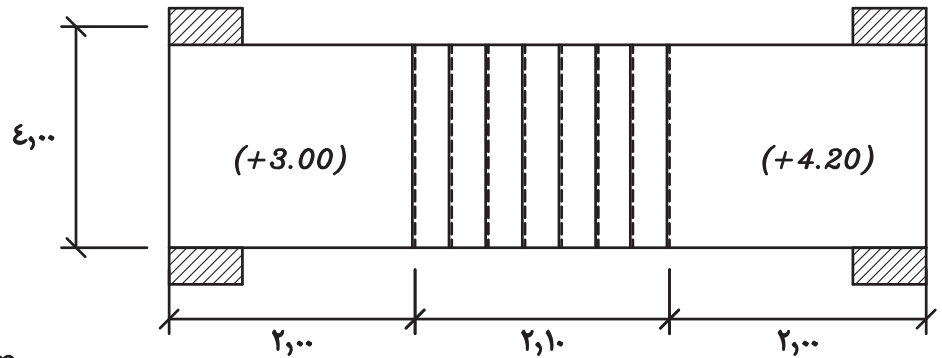
$$F.C. = 1.50 \text{ kN/m}^2$$

$$L.L. = 3.00 \text{ kN/m}^2$$

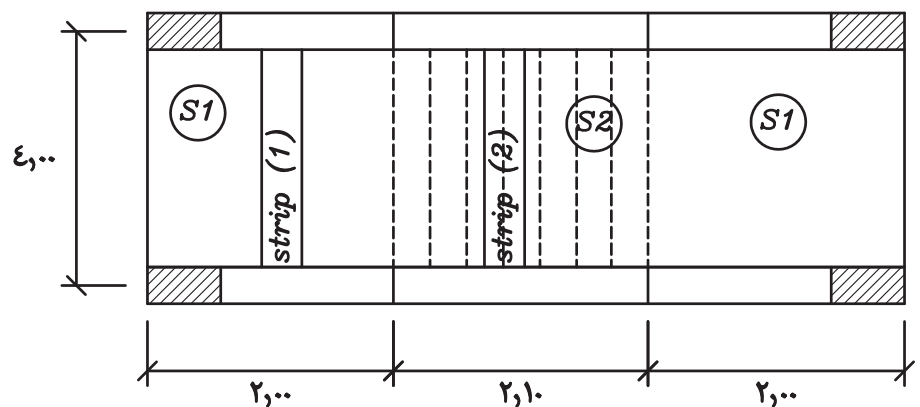
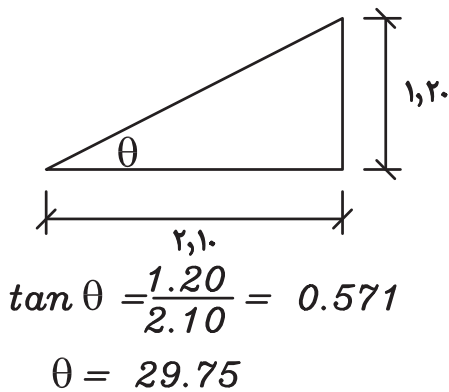
$$f_{cu} = 25 \text{ N/mm}^2$$

Steel used 360/520

$$O.w \text{ of beam} = 4.0 \text{ kN/m}$$



Solution



1-slab thickness

for one way slab S1

$$t_s = \frac{L}{25} = \frac{400}{25} = 16 \text{ cm}$$

for one way slab S2

$$t_s = \frac{L}{25} = \frac{400}{25} = 16 \text{ cm}$$

⇒ Take $t_s = 16 \text{ cm}$

$$t_{av} = t_s + 7 = 23 \text{ cm}$$

2-Calculation of load

For Landing :-

$$w_{shz} = 1.4[t_s \gamma_c + F.c.] + 1.6L.L. \quad \text{kN/m}^2$$

$$w_{shz} = 1.4[0.16*25 + 1.50] + 1.6*3.00$$

$$w_{shz} = 12.50 \text{ kN/m}^2$$

For Flight :-

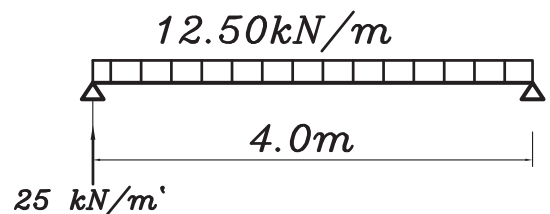
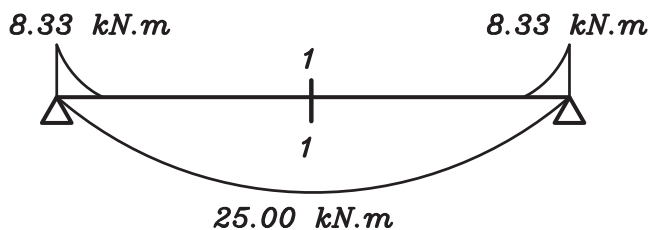
$$w_{sin} = 1.4[t_{sav} \gamma_c + F.c.] + 1.6L.L \cos \theta \quad \text{kN/m}^2$$

$$w_{sin} = 1.4[0.23*25 + 1.50] + 1.6*3.00*\cos(29.75)$$

$$w_{sin} = 14.32 \text{ kN/m}^2$$

-Design of sections

Strip (1) :-



Sec(1-1)

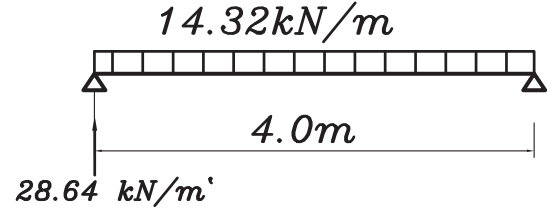
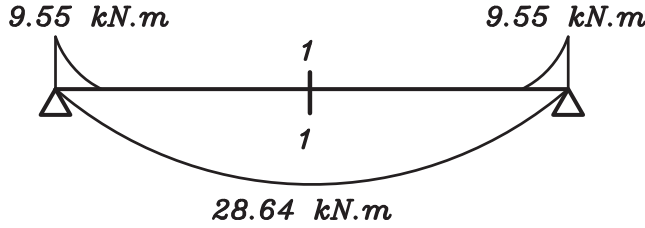
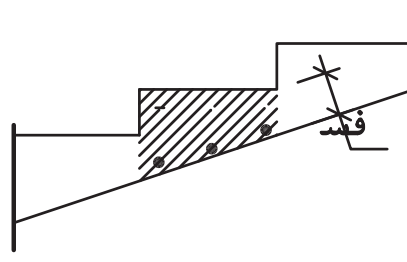
$$M_{u.l.} = 25.00 \text{ kN.m} \quad \& \quad B = 1000 \text{ mm} \quad \& \quad d = 160 - 20 = 140 \text{ mm}$$

$$140 = C_1 \sqrt{\frac{25.00 * 10^6}{1000 * 25}} \quad C_1 = 4.43 \quad \& \quad J = 0.82$$

$$A_s = \frac{25.00 * 10^6}{0.82 * 140 * 360} = 607.58 \text{ mm}^2/\text{m}'$$

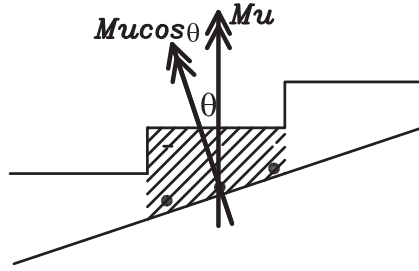
$$A_s = \underline{6 \phi 12/\text{m}'}$$

Strip (2) :-



- ملحوظة

لاحظ أننا هنا نعتبر هذه الشريحة أفقية في بلاطة مائلة لأن العزم موجب وماثل على أتجاه الحديد لذلك فهو يحتاج الى تحليل



Sec(1-1)

$$M_{u.l.} = 28.64 \text{ kN.m} \quad \& \quad B = 1000 \text{ mm} \quad \& \quad d = 160 - 20 = 140 \text{ mm}$$

$$M_{des} = 28.64 \cos \theta = 24.87 \text{ kN.m}$$

$$140 = C_1 \sqrt{\frac{24.87 * 10^6}{1000 * 25}} \quad C_1 = 4.44 \quad \& \quad J = 0.82$$

$$A_s = \frac{24.87 * 10^6}{0.82 * 140 * 360} = 604.20 \text{ mm}^2/\text{m}'$$

$$A_s = \underline{6 \phi 12/\text{m}'}$$

Details of R.F.T. for Stair

